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State of West Virginia Department of Administration Purchasing Division 2019 Washington Street East Post Office Box 50130 Charleston, WV 25305-0130

Request for Quotation

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ADDRESS	CORRESPO	DNDENCET	OATTENT	ION OF:

JOHN ABBOTT 304-558-2544

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DIV ENGINEERING & FACILITIES CAMP DAWSON ARMY TRAINING SITE 240 ARMY ROAD

KINGWOOD, WV 26537-1077

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GENERAL TERMS & CONDITIONS REQUEST FOR QUOTATION (RFQ) AND REQUEST FOR PROPOSAL (RFP)

- 1. Awards will be made in the best interest of the State of West Virginia.
- 2. The State may accept or reject in part, or in whole, any bid.
- 3. All quotations are governed by the West Virginia Code and the Legislative Rules of the Purchasing Division.
- 4. Prior to any award, the apparent successful vendor must be properly registered with the Purchasing Division and have paid the required \$125 fee.
- 5. All services performed or goods delivered under State Purchase Order/Contracts are to be continued for the term of the Purchase Order/Contracts, contingent upon funds being appropriated by the Legislature or otherwise being made available. In the event funds are not appropriated or otherwise available for these services or goods, this Purchase Order/Contract becomes void and of no effect after June 30.
- Payment may only be made after the delivery and acceptance of goods or services.
- 7. Interest may be paid for late payment in accordance with the West Virginia Code.
- 8. Vendor preference will be granted upon written request in accordance with the West Virginia Code.
- 9. The State of West Virginia is exempt from federal and state taxes and will not pay or reimburse such taxes.
- 10. The Director of Purchasing may cancel any Purchase Order/Contract upon 30 days written notice to the seller.
- **11.** The laws of the State of West Virginia and the *Legislative Rules* of the Purchasing Division shall govern all rights and duties under the Contract, including without limitation the validity of this Purchase Order/Contract.
- 12. Any reference to automatic renewal is hereby deleted. The Contract may be renewed only upon mutual written agreement of the parties.
- 13. BANKRUPTCY: In the event the vendor/contractor files for bankruptcy protection, this Contract may be deemed null and void, and terminated without further order.
- 14. HIPAA BUSINESS ASSOCIATE ADDENDUM: The West Virginia State Government HIPAA Business Associate Addendum (BAA), approved by the Attorney General, and available online at the Purchasing Division's web site (http://www.state.wv.us/admin/purchase/vrc/hipaa.htm) is hereby made part of the agreement. Provided that, the Agency meets the definition of a Cover Entity (45 CFR §160.103) and will be disclosing Protected Health Information (45 CFR §160.103) to the vendor.
- 15. WEST VIRGINIA ALCOHOL & DRUG-FREE WORKPLACE ACT: If this Contract constitutes a public improvement construction contract as set forth in Article 1D, Chapter 21 of the West Virginia Code ("The West Virginia Alcohol and Drug-Free Workplace Act"), then the following language shall hereby become part of this Contract: "The contractor and its subcontractors shall implement and maintain a written drug-free workplace policy in compliance with the West Virginia Alcohol and Drug-Free Workplace Act, as set forth in Article 1D, Chapter 21 of the West Virginia Code. The contractor and its subcontractors shall provide a sworn statement in writing, under the penalties of perjury, that they maintain a valid drug-free work place policy in compliance with the West Virginia and Drug-Free Workplace Act. It is understood and agreed that this Contract shall be cancelled by the awarding authority if the Contractor: 1) Fails to implement its drug-free workplace policy; 2) Fails to provide information regarding implementation of the contractor's drug-free workplace policy at the request of the public authority; or 3) Provides to the public authority false information regarding the contractor's drug-free workplace policy."

INSTRUCTIONS TO BIDDERS

- 1. Use the quotation forms provided by the Purchasing Division.
- **2. SPECIFICATIONS:** Items offered must be in compliance with the specifications. Any deviation from the specifications must be clearly indicated by the bidder. Alternates offered by the bidder as **EQUAL** to the specifications must be clearly defined. A bidder offering an alternate should attach complete specifications and literature to the bid. The Purchasing Division may waive minor deviations to specifications.
- 3. Complete all sections of the quotation form.
- 4. Unit prices shall prevail in case of discrepancy.
- 5. All quotations are considered F.O.B. destination unless alternate shipping terms are clearly identified in the quotation.
- **6. BID SUBMISSION:** All quotations must be delivered by the bidder to the office listed below prior to the date and time of the bid opening. Failure of the bidder to deliver the quotations on time will result in bid disqualifications: Department of Administration, Purchasing Division, 2019 Washington Street East, P.O. Box 50130, Charleston, WV 25305-0130



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State of West Virginia
Department of Administration
Purchasing Division
2019 Washington Street East Post Office Box 50130 Charleston, WV 25305-0130

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DIV ENGINEERING & FACILITIES CAMP DAWSON ARMY TRAINING SITE

SH-P 240 ARMY ROAD

KINGWOOD, WV 26537-1077

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DIV ENGINEERING & FACILITIES CAMP DAWSON ARMY TRAINING SITE 240 ARMY ROAD

KINGWOOD, WV 26537-1077

304-329-4417

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State of West Virginia
Department of Administration
Purchasing Division
2019 Washington Street East
Post Office Box 50130 Charleston, WV 25305-0130

Request for Quotation

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JOHN ABBOTT 304-558-2544

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KINGWOOD, WV 26537-1077

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State of West Virginia
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JOHN ABBOTT 304-558-2544

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DIV ENGINEERING & FACILITIES CAMP DAWSON ARMY TRAINING SITE 240 ARMY ROAD

KINGWOOD, WV 26537-1077

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DIV ENGINEERING & FACILITIES CAMP DAWSON ARMY TRAINING SITE 240 ARMY ROAD

KINGWOOD, WV 26537-1077

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JOHN	ABBOTT
304-5	58-2544

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DIV ENGINEERING & FACILITIES OT-P CAMP DAWSON ARMY TRAINING SITE 240 ARMY ROAD T O

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Project Name:	WVARNG South Gate
Road	
Project No. 7-	7728-0000

By: <u>CJR</u> Chk'd By: <u>DWD</u>

WVARNG South Gate Road Slope Repair Design Brief

Dated: 2/25/09

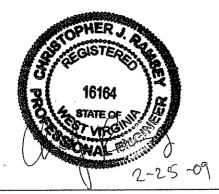
Prepared For:

WVARNG



Prepared By:

amec[©]



File: South Gate Road Design

Brief.xmcd

Page 1 of 11

Date: 12/04/08



Projec	ct Name	: WVAKNO	i South	Gate
Road			4	
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By: _	CJR			
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Date: 12/04/08

Design Discussion

The South Gate Access Road has suffered from intermittent erosion and minor landslides, which have led to a state of disrepair and limited use. AMEC prepared three concepts for stabilization of approximately 230 linear feet of the road. AMEC recommended, and WVARNG concurred that the best method of repair was a drilled shaft wall embedded into bedrock with precast concrete lagging spanning between the shafts near the surface. Multiple exploratory borings and surface reconnaissance were used to estimate the limits of the proposed drilled shaft retaining wall, the loads that would be induced on the system as it supports the restored road grade and adjacent hillside, and the soil & rock properties within the underlying subsurface profile.

Lab tests indicate an average unit weight for the overburden soils is approximately 115 pounds per cubic foot (pcf). Considering the amount of clay content in soil matrix, an appropriate friction angle would be 25 degrees. Field and lab inspection of the rock core samples indicate a hard shale, that would most likely not be easily removed with normal augering tools. As such, we assigned a unit weight of 140 pcf, a friction angle of 40 degrees and a shear strength of 10,000 pounds per square foot to the rock

Drilled shaft retaining walls are common in roadsides next to river valley applications, similar to this. The methodology developed to design this system is derived from the following sources:

- AASHTO Standard Specifications for Bridges, 17th Edition
- FHWA Publication Geotechnical Engineering Circular No. 4 (1999), and
- "Slide Control by Drilled Pier Walls", M. Nethero, ASCE National Convention; Las Vegas, NV; 1982.



Project Name: <u>WVARNG South Gate</u>

Road

Project No. 7-7728-0000

By: CJR

DWD Chk'd By: _

Drilled Shaft Retaining Wall - Design Section 11+00:

Soil Parameters:

Unit weight

 $\gamma_{\text{soil}} := 115 \cdot \text{pcf}$

Phi angle

 $\phi_{soil} := 25 \cdot deg$

 $k_{a_soil} := \tan\left(45 \cdot \deg - \frac{\varphi_{soil}}{2}\right)^{2}$ $k_{a_soil} := \frac{1}{k_{a_soil}}$ $k_{p_soil} := 2.46$

Factor of Safety on Passive

FS := 1.5

Rock Parameters:

Unit weight

 $\gamma_{rock} := 140 \cdot pcf$ Phi angle $\varphi_{rock} := 40 \cdot deg$

 $k_{a_rock} := tan \left(45 \cdot deg - \frac{\Phi_{rock}}{2} \right)^2$

Factor of Safety on Passive

FS := 1.5

 $k_{p_rock} := \frac{1}{k_{a_rock}}$ $k_{p_rock} = 4.60$

Rock Shear Strength

 $C_{rock} := 10 ksf$

Wall Characteristics:

Wall height

 $H := 27 \cdot ft$

Depth of Overburden h := 18ft

Pile Width

 $b_f := 2.5 \cdot ft$ Pile spacing

 $s := 10 \cdot ft$

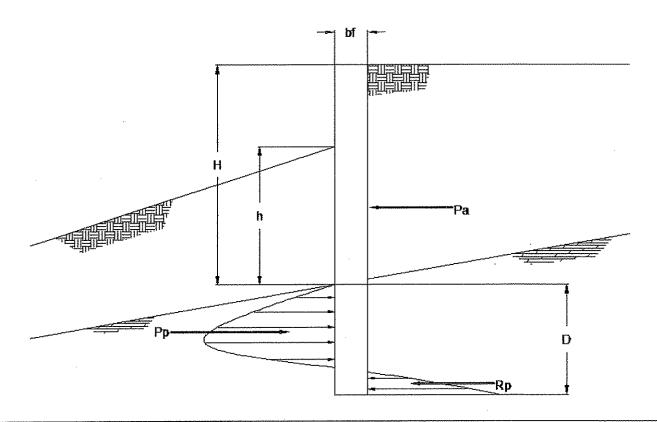
Horizontal Surcharge Pressure applied

q := 100psf

 $b_e := if(3 \cdot b_f < s, 3 \cdot b_f, s)$ $b_e = 7.5 ft$

Depth of Rock Socket

D := 12ft



File: South Gate Road Design

Brief.xmcd

Page 3 of 11

Date: 12/04/08



Project Name: <u>WVARNG South Gate</u>
Road
Project No. _7-7728-0000

By: <u>CJR</u>

Chk'd By: __DWD

$$P_a := k_{a_soil} \cdot \gamma_{soil} \cdot \frac{(H)^2}{2} = 17.01 \cdot \frac{kips}{ft}$$

SUM MOMENTS ABOUT "P" AND SOLVE FOR "Rp"

$$R_{p} := \left[\left(P_{a} \cdot s \right) \cdot \left(\frac{H}{3} + \frac{D}{3} \right) - \gamma_{soil} \cdot \left[\frac{k_{p_soil} \cdot b_{e}}{FS} \cdot \left[\frac{\left(h \right)^{2}}{2} \right] \right] \cdot \left(\frac{h}{3} + \frac{D}{3} \right) + q \cdot H \cdot s \cdot \left(\left(\frac{H}{2} + \frac{D}{3} \right) \right) \right] \cdot \frac{9}{D \cdot 5} = 58.35 \cdot \text{kips}$$

SUM OF THE HORIZONTAL FORCES = 0, DETERMINE "Pp"

$$P_p := P_a \cdot s - \left[\gamma_{soil} \cdot \frac{k_{p_soil} \cdot b_e}{FS} \cdot \left(\frac{h^2}{2} \right) \right] + q \cdot H \cdot s + R_p = 25.96 \cdot kips$$

$$P_{p_avg} := \frac{P_p}{2 \cdot D \cdot b_f} = 1.3 \cdot ksf$$

$$P_{p_{max}} := 1.5 \cdot P_{p_{avg}} = 1.95 \cdot ksf$$

DUE TO PARABOLIC DISTRIBUTION

$$R_{p_avg} := \frac{R_p}{D \cdot \frac{b_f}{3}} = 5.83 \cdot ksf$$

$$R_{p_max} \coloneqq 2 \cdot R_{p_avg} = 11.67 \cdot ksf$$

DUE TO TRIANGULAR DISTRIBUTION



1 4 Project Name: <u>WVARNG South Gate</u> Road Project No. _7-7728-0000

CJR

OK!

Chk'd By:

ULTIMATE PASSIVE PRESSURE @ Pp or D/3

$$D3_{pass} := \gamma_{soil} \cdot H + 2 \cdot C_{rock} + \gamma_{rock} \cdot \frac{D}{3} = 23.66 \cdot ksf$$

$$FS_{D3_pass} := \frac{D3_{pass}}{P_{p_max}} = 12.15$$
 > 2.0

ULTIMATE PASSIVE PRESSURE @ Rp or 8D/9

$$D_{8_9pass} := \gamma_{soil} \cdot H + 2 \cdot C_{rock} + \gamma_{rock} \cdot \frac{8D}{9} = 24.6 \cdot ksf$$

$$FS_{D8_9pass} := \frac{D_{8_9pass}}{R_{p_avg}} = 4.22$$
 > 2.0 OK!

ULTIMATE PASSIVE PRESSURE @ D

$$D_{\text{pass}} := \gamma_{\text{soil}} \cdot H + 2 \cdot C_{\text{rock}} + \gamma_{\text{rock}} \cdot D = 24.79 \cdot \text{ksf}$$

$$FS_{Dpass} := \frac{D_{pass}}{R_{p, max}} = 2.12$$
 > 2.0 OK!

FIND POINT OF ZERO SHEAR

$$V_o := \frac{\left[P_a \cdot s - \left[\gamma_{soil} \cdot \frac{k_{p_soil} \cdot b_e}{FS} \cdot \left(\frac{h^2}{2}\right)\right] + q \cdot H \cdot s\right]}{b_f \cdot P_{p_avg}} = -9.98 \, \text{ft} \qquad \text{ABOVE TOP OF ROCK}$$

MAXIMUM MOMENT

$$M_{max} := \left(\frac{H}{3} + V_o\right) \cdot P_a \cdot s - \left(\frac{h}{3} + V_o\right) \cdot \left[\gamma_{soil} \cdot \frac{k_{p_soil} \cdot b_e}{FS} \cdot \left(\frac{h^2}{2}\right)\right] + \left(\frac{H}{2} + V_o\right) \cdot q \cdot H \cdot s - P_{p_avg} \cdot b_f \cdot \frac{{V_o}^2}{2} = 680.16 \cdot ft \cdot kips$$

$$F_{v} := 50 \text{ksi}$$
 $F_{a} := .66 F_{v} = 33 \cdot \text{ksi}$

$$S_{x_rqd} := \frac{M_{max}}{F_o} = 247.33 \cdot in^3$$
 USE W21x111 GR50 PILE



Project Name: _WVARNG South Gate

Road

Project No. _7-7728-0000

<u>CJR</u>

DWD Chk'd By:

Drilled Shaft Retaining Wall - Design Sections 12+00 & 13+00:

Soil Parameters:

Unit weight
$$\gamma_{soil} := 115 \cdot pcf$$

$$\phi_{\text{soil}} := 25 \cdot \text{deg}$$

$$\phi_{\text{soil}} := 25 \cdot \text{deg}$$
 $k_{\text{a_soil}} := \tan \left(45 \cdot \text{deg} - \frac{\phi_{\text{soil}}}{2} \right)^2$
 $k_{\text{a_soil}} := \frac{1}{k_{\text{a_soil}}}$
 $k_{\text{p_soil}} := 2.46$

$$k_{a_soil} = 0.41$$

$$FS = 1.5$$

$$k_{p_soil} := \frac{1}{k_{a_soil}}$$

$$k_{p_soil} = 2.46$$

Rock Parameters:

$$\gamma_{rock} \coloneqq 140 \cdot pcf \quad \text{ Phi angle } \quad \varphi_{rock} \coloneqq 40 \cdot deg$$

$$\phi_{\text{rock}} := 40 \cdot \text{deg}$$

$$k_{a_rock} := tan \left(45 \cdot deg - \frac{\phi_{rock}}{2} \right)^2$$

$$k_{a_rock} = 0.22$$

$$FS := 1.5$$

$$k_{p_rock} := \frac{1}{k_{a_rock}} \qquad k_{p_rock} = 4.60$$

$$k_{p_rock} = 4.60$$

$$C_{rock} := 10ksf$$

Wall Characteristics:

Wall height

 $H := 20 \cdot ft$

Depth of Overburden h := 11ft

Pile Width

 $b_f := 2.5 \cdot ft$ Pile spacing

 $s := 10 \cdot ft$

Horizontal Surcharge Pressure applied

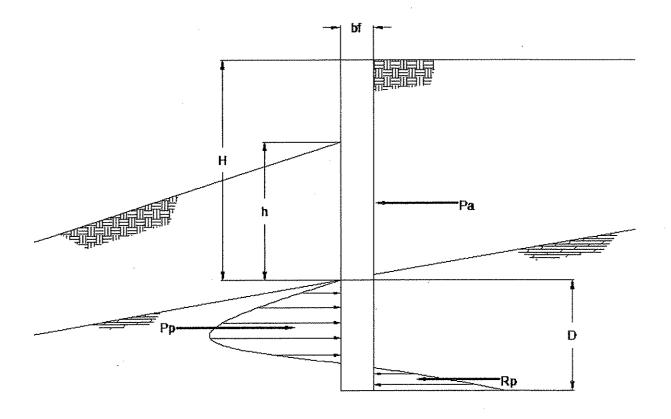
$$q := 100psf$$

$$b_e := if(3 \cdot b_f < s, 3 \cdot b_f, s)$$
 $b_e = 7.5 ft$

$$b = 7.5 ft$$

Depth of Rock Socket

$$D := 15ft$$



File: South Gate Road Design

Brief.xmcd

Page 6 of 11

Date: 12/04/08



Project Name: <u>WVARNG South Gate</u>
Road
Project No. _7-7728-0000

By: <u>CJR</u> Chk'd By: <u>DWD</u>

$$P_a := k_{a_soil} \cdot \gamma_{soil} \cdot \frac{(H)^2}{2} = 9.33 \cdot \frac{kips}{ft}$$

SUM MOMENTS ABOUT "P" AND SOLVE FOR "Rp"

$$R_{p} := \left[\left(P_{a} \cdot s \right) \cdot \left(\frac{H}{3} + \frac{D}{3} \right) - \gamma_{soil} \cdot b_{e} \cdot \left[\frac{k_{p_soil}}{FS} \cdot \left[\frac{\left(h \right)^{2}}{2} \right] \right] \cdot \left(\frac{h}{3} + \frac{D}{3} \right) + q \cdot H \cdot s \cdot \left(\left(\frac{H}{2} + \frac{D}{3} \right) \right) \right] \cdot \frac{9}{D \cdot 5} = 77.54 \cdot \text{kips}$$

SUM OF THE HORIZONTAL FORCES = 0, DETERMINE "Pp"

$$P_{p} := P_{a} \cdot s - \left[\gamma_{soil} \cdot b_{e} \cdot \frac{k_{p_soil}}{FS} \cdot \left(\frac{h^{2}}{2} \right) \right] + q \cdot H \cdot s + R_{p} = 105.18 \cdot kips$$

$$P_{p_avg} := \frac{P_p}{\frac{2 \cdot D \cdot b_f}{3}} = 4.21 \cdot ksf$$

$$P_{p_max} := 1.5 \cdot P_{p_avg} = 6.31 \cdot ksf$$

$$R_{p_avg} := \frac{R_p}{D \cdot \frac{b_f}{3}} = 6.2 \cdot ksf$$

$$R_{p_max} := 2 \cdot R_{p_avg} = 12.41 \cdot ksf$$

DUE TO TRIANGULAR DISTRIBUTION

Date: 12/04/08



Project Name: <u>WVARNG South Gate</u>
Road

Project No. <u>7-7728-0000</u>

By: ___CJF

Chk'd By: DWD

ULTIMATE PASSIVE PRESSURE @ Pp or D/3

$$D3_{pass} := \gamma_{soil} \cdot H + 2 \cdot C_{rock} + \gamma_{rock} \cdot \frac{D}{3} = 23 \cdot ksf$$

$$FS_{D3_pass} := \frac{D3_{pass}}{P_{p_{max}}} = 3.64$$

2.0

OK!

ULTIMATE PASSIVE PRESSURE @ Rp or 8D/9

$$D_{8_9pass} := \gamma_{soil} \cdot H + 2 \cdot C_{rock} + \gamma_{rock} \cdot \frac{8D}{9} = 24.17 \cdot ksf$$

$$FS_{D8_9pass} := \frac{D_{8_9pass}}{R_{p_avg}} = 3.9$$

2.0

OK!

ULTIMATE PASSIVE PRESSURE @ D

$$D_{pass} := \gamma_{soil} \cdot H + 2 \cdot C_{rock} + \gamma_{rock} \cdot D = 24.4 \cdot ksf$$

$$FS_{Dpass} := \frac{D_{pass}}{R_{p max}} = 1.97$$

2.0

OK!

FIND POINT OF ZERO SHEAR

$$V_o := \frac{\left[P_{a} \cdot s - \left[\gamma_{soil} \cdot b_e \cdot \frac{k_{p_soil}}{FS} \cdot \left(\frac{h^2}{2}\right)\right] + q \cdot H \cdot s\right]}{b_f \cdot P_{p_avg}} = 2.63 \, ft$$

ABOVE TOP OF ROCK

MAXIMUM MOMENT

$$M_{max} := \left(\frac{H}{3} + V_o\right) \cdot P_a \cdot s - \left(\frac{h}{3} + V_o\right) \cdot \left[\gamma_{soil} \cdot b_e \cdot \frac{k_{p_soil}}{FS} \cdot \left(\frac{h^2}{2}\right)\right] + \left(\frac{H}{2} + V_o\right) \cdot q \cdot H \cdot s - P_{p_avg} \cdot b_f \cdot \frac{{V_o}^2}{2} = 544.34 \cdot ft \cdot kips$$

$$F_{\nu} := 50 \text{ks}$$

$$F_a := .66F_y = 33 \cdot ksi$$

$$S_{x_rqd} := \frac{M_{max}}{F_a} = 197.94 \cdot in^3$$

USE W18x106 GR50 PILE



Project Name: _WVARNG South Gate Road

Project No. _7-7728-0000

Chk'd By:

Precast Lagging Design:

$$w := \frac{P_{a_max}}{H} + q = 950.5 \cdot psf$$

Clear Span of Lagging ---> $S_{clr} := s - 2t_{brg} = 9.17 \, ft$

$$S_{clr} := s - 2t_{brg} = 9.17 \, ft$$

$$t_{\text{brg}} \coloneqq 5 \text{in}$$

$$M_{\text{max_lag}} := w \cdot \frac{S_{\text{clr}}^2 \cdot 1 \, \text{ft}}{10} = 7.99 \cdot \text{ft-kips}$$

$$V_{\text{max_lag}} := w \cdot \frac{S_{\text{clr}} \cdot 1 \, \text{ft}}{2} = 4.36 \cdot \text{kips}$$

TRY 8" THICK LAGGING

$$f_c := 4ksi$$

$$t_{lag} := 8in$$

$$f_v := 60 \text{ksi}$$

$$d := t_{lag} - 2in = 6 \cdot in$$

$$d_{bf} := \frac{6}{8} \cdot in$$

$$A_{bf} := \frac{\pi \cdot d_{bf}^2}{4}$$
 $A_{bf} = 0.44 \cdot in^2$

$$A_{\rm bf} = 0.44 \cdot \text{in}^2$$

$$N := 4$$

Spacing :=
$$\frac{(3ft)}{N}$$

Spacing =
$$9 \cdot in$$

$$As := \frac{(12in)}{Spacing} A_{bf} \qquad As = 0.59 \cdot in^2$$

$$As = 0.59 \cdot in^2$$

MOMENT CALCULATION

$$T_{lag} := As \cdot f_y = 35.34 \cdot kips$$

$$a:=\frac{T_{lag}}{0.85\cdot f_c\cdot b}=0.87\cdot in$$

$$M := T_{lag} \cdot \left(d - \frac{a}{2} \right) = 196.75 \cdot in \cdot kips$$

$$M_u := M_{ma}$$

$$M_u := M_{\text{max_lag}} \cdot \frac{1.7}{0.9} = 181.04 \cdot \text{in kips}$$



Project Name: <u>WVARNG South Gate</u>

Road

Project No. <u>7-7728-0000</u>

By: <u>CJR</u> Chk'd Bv:

DWD

BALANCED REINFORCEMENT RATIO

$$\rho b := \frac{0.85 \cdot \beta_1 \cdot f_c}{f_v} \cdot \frac{87000 \cdot psi}{87000 \cdot psi + f_v}$$

$$\rho b = 0.029$$

Balanced condition

Asmax := $0.75 \cdot \rho b \cdot b \cdot d$ Asmax = $1.54 \cdot in^2$

ACI 10.3.3 "As" provided for flexure must be less than this to ensure ductile behavior <----

$$\rho min := \frac{200 \cdot psi}{f_y}$$

ACI 10.5 minimum reinforcement required, Alternatively, area of reinforcement provided at every section shall be at least 1/3 greater than that required by analysis. <----

Asmin := ρ min b d Asmin = $0.24 \cdot in^2$

 $a := \frac{As \cdot f_y}{\beta_1 \cdot f_x \cdot b} \qquad a = 0.87 \cdot in$

. . . . 2

 $s = 0.59 \cdot in^2$ <--- check with above Asmax and Asmin criteria

SHEAR CHECK

$$V_n := 2\sqrt{f_c} \cdot b \cdot d = 0.13 \text{ ft}^{0.5} \cdot s \cdot 1b^{-0.5} \cdot \text{kips}$$

$$V_u := V_{\text{max_lag}} \cdot \frac{1.7}{0.85} = 8.71 \cdot \text{kips}$$

OK!

BEARING

$$P_{lag} := w \cdot \frac{S_{clr} \cdot 1ft}{2} = 4.36 \cdot kips$$

$$A_b := \frac{(P_{lag} \cdot 1.7)}{(0.7) \cdot 0.85 \cdot f_* \cdot 12in} = 0.26 \cdot in$$

MIN. BEARING > .260" PER SIDE, OR .347" WITH A 0.75 FACTOR

FOR 3 FEET TALL PANELS, USE 4-#6 BARS SPACED AT 9" HORIZ., WITH 3" CLEAR AND 4-#4 BARS VERTICAL SPACED EQUALLY ACROSS, WITH 3" CLEAR



Project Name: <u>WVARNG South Ga</u> Road	<u>ite</u>
Project No. <u>7-7728-0000</u>	
By: CJR	
Chk'd By: DWD	

20

APPENDIX A SOIL BORING LOGS

	BORING NUM
Bluegrass Parkway Suite 690	

CLIENT _Army National Guard PROJECT NAME _South Gate Road Slope Failure PROJECT NUMBER _ 7-7728-0000-0002 PROJECT LOCATION _Camp Dawson, West Vir DATE STARTED _ 10/10/08					
PROJECT NUMBER 7-7728-0000-0002 PROJECT LOCATION Camp Dawson, West Vir	rginia				
DATE STARTED 10/10/08 COMPLETED 10/10/08 GROUND ELEVATION 1280.3 ft HOLE S					
	SIZE	3.25	<u>.</u>		
DRILLING CONTRACTOR MATHES GROUND WATER LEVELS:					
DRILLING METHOD HSA AT TIME OF DRILLING					
LOGGED BY MGS AT END OF DRILLING			····		***************************************
NOTES Dry Hole AFTER DRILLING					
SAMPLE TYPE NUMBER NUMBER NUMBER (RQD) BLOW COUNTS (N VALUE) POCKET PEN. (RSD) DRY UNIT WT. (RS) (RS)	MOISTURE CONTENT (%)	LIGUID	PLASTIC WELL	S	FINES CONTENT
	28		<u> </u> 4	PLA =	Ē
0 NO SAMPLE POSSIBLE DUE TO PRESENCE OF RIP RAP		1		1	
GRAVEL, FINE, WITH SILTY CLAY, LOOSE		- Company			
5 ROCK, HIGHLY WEATHERED, SOME SILTY CLAY, DARK					
GRAY ST 2 100	21	48	28	20	29
CLAY, SILTY, WITH FINE TO COARSE ROCK FRAGMENTS, SOME WEATHERED SHALE, BROWN, STIFF SS 3 2-3-9 (12) SS 3 2-3-9 (12) SS 4 EACH OF THE STIPLE SILTY CLAY WITH FINE TO	100000	AVUIS TITLE			
15 SS 4 22-40-47 (87)					
WEATHERED SHALE, LITTLE SILTY CLAY WITH FINE TO COARSE SAND, DARK GRAY, VERY DENSE SHALE, LIGHT GRAY 50/1"					
20 17.3-17.5, 17.8-18.0 - HIGHLY WEATHERED ZONES 6 (88) FRACTURE					
26.4-26.5, 27.0 - MODERATELY WEATHERED FRACTURE 27.3 - SLIGHTLY WEATHERED FRACTURE 27.5 - SLIGHT TO MODERATELY WEATHERED FRACTURE 28.3-28.5, 28.7, 28.9, 29.5, 29.6, 30.1, 31.1 - SLIGHTLY WEATHERED FRACTURE 7 (100)					
25					
30 RC 8 (50)					
COARSE SAND, DARK GRAY, VERY DENSE SHALE, LIGHT GRAY 17.3-17.5, 17.8-18.0 - HIGHLY WEATHERED ZONES 18.6, 19.3, 21.6, 23.5, 24.5 - SLIGHTLY WEATHERED FRACTURE 26.4-26.5, 27.0 - MODERATELY WEATHERED FRACTURE 27.3 - SLIGHTLY WEATHERED FRACTURE 27.5 - SLIGHT TO MODERATELY WEATHERED FRACTURE 28.3-28.5, 28.7, 28.9, 29.5, 29.6, 30.1, 31.1 - SLIGHTLY WEATHERED FRACTURE 25 RC 100 7 (100) RC 6 (88) RC 100 9 (91)					
Refusal at 18.3 feet. Bottom of borehole at 33.7 feet.					

BORING NUMBER E-4

AMEC

ame	H.CO	11003 Bluegrass Parkway Suite 690 Louisville, KY 40299 502-267-0700									PAGE	i Or	- 1
CLIENT	Γ <u>Arm</u>	y National Guard											
PROJE	CT NU	MBER 7-7728-0000-0002	PROJECT										
DATES	STARTI	ED 10/9/08 COMPLETED 10/9/08	GROUND	ELEVAT	TON _	1273.7 ft		HOLE	SIZE	3.25			
DRILLI	NG CO	NTRACTOR MATHES	GROUND	WATER	LEVEL	LS:							
DRILLI	NG ME	THOD HSA				ING							
LOGGE	ED BY	MGS				ING	n		·····				
NOTES	Dry	Hole	AF	TER DRII	LLING					ATT	ERBE	RG 1	
o DEPTH	GRAPHIC LOG	MATERIAL DESCRIPTION		SAMPLE TYPE NUMBER	RECOVERY % (RQD)	BLOW COUNTS (N VALUE)	POCKET PEN. (tsf)	DRY UNIT WT. (pcf)	MOISTURE CONTENT (%)	LIQUID	PLASTIC WIT	PLASTICITY (FINES CONTENT
		GRAVEL, FINE, WITH LITTLE SILTY CLAY AND FINE T COARSE SAND, LOOSE		SS 1		4-4-5 (9)							
5		SILT, CLAYEY, LITTLE FINE TO COARSE SAND, LITTI GRAVEL, TAN AND GRAY, STIFF		SS 2		5-6-9 (15)							
		CLAYEY SILT, TAN AND REDDISH BROWN, VERY ST	(PT	SS 3	1	15-9-14 (23)		Name of the Party		- Land William	-		
10		WEATHERED SHALE, DARK GRAY, VERY STIFF		SS 4		4-7-10 (17)				Attitude of the state of the st	The state of the s	William Company	
15		CLAY, SILTY, WITH ROCK FRAGMENTS (HIGHLY WEATHERED), HARD		SS 5	-	50/3"							
15	<u> </u>	Refusal at 15.0 feet. Bottom of borehole at 15.0 feet.											
GEOTECH BH COLUMNS - GINT STD US LAB.GDT - 12/17/08 08:38 - N./GINTVPROJECTSVARNG SL				iles, managana ma									
EOTECH BH C			manufik til för fra til en sed (f. f.)		ay and a pundhish	—— suu maannadohiniighallik ka ad	uu amaneera ka a a f	one of the halfa					

AMEC 11003 E

Bluegrass Parkway Suite 690		

BORING NUMBER E-5

PAGE	1	OF	1

am	ec"	Louisville, KY 40299 502-267-0700		-								. •	
CLIEN	er <u>Arr</u>	ny National Guard	PROJEC	T NAME	South	Gate Roa	d Slop	e Faile	ıre				
		UMBER _7-7728-0000-0002	PROJEC	T LOCATI	ON_	Camp Daw	son, V	/est V	irginia				
DATE	STAR	TED 10/9/08 COMPLETED 10/9/08	GROUND	ELEVAT	ON_	1271.2 ft		HOLE	SIZE	3.25			
DRILL	JNG C	ONTRACTOR MATHES	GROUND	WATER	LEVE	LS:							
DRILL	JING M	ETHOD HSA	AT	TIME OF	DRILI	LING							
LOGG	RED BY	MGS	AT	END OF	DRILL	JNG							
NOTE	S Dn	Hole	AF	TER DRIL	LING								
				Й	%		ż	T.	(9	ATT	ERBE	RG	Z
o DEPTH	GRAPHIC LOG	MATERIAL DESCRIPTION		SAMPLE TYP NUMBER	RECOVERY (RQD)	BLOW COUNTS (N VALUE)	POCKET PEN. (tsf)	DRY UNIT WT. (pcf)	MOISTURE CONTENT (9		PLASTIC LIMIT	PLASTICITY INDEX	FINES CONTENT (%)
<u> </u>		SILT, CLAYEY, MIX OF GRAY AND REDDISH BROWN,											
		MEDIUM STIFF		X SS 1		4-4-3 (7)							
5	2000	SILT, CLAYEY, TRACE FINE GRAVEL, LITTLE BLACK NODULES, BROWN, MEDIUM STIFF		SS 2		5-4-4 (8)							
		SILT, CLAYEY, TRACE BLACK NODULES, TAN, VERY S	STIFF	SS 3		6-11-11 (22)							
10				SS 4		8-9-11 (20)				ANTANAMAN UTFFTATOR TT			
 _ 15		SILT, CLAYEY, LITTLE FINE TO COARSE ROCK FRAGI TAN, VERY STIFF	MENTS,	SS 5	***************************************	15-12-12 (24)			Addition designation of the state of the sta	The statement and an area and a statement and		***************************************	
20		SHALE, LIGHT GRAY 18.7, 19.6 - SLIGHTLY WEATHERED FRACTURE 20.0 - SLIGHT TO MODERATELY WEATHERED FRACT 20.3-21.2, 21.7-21.9 - ROCK FRAGMENTS, SLIGHT TO MODERATELY WEATHERED 22-23 ZONE OF LOSS 23-23.5 ROCK FRAGMENTS SLIGHT TO MODERATELY		RC 6	100 (72)				THE				
25		WEATHERED 23.8 MODERATELY WEATHERED FRACTURE 24.3-24.5 ROCK FRAGMENTS SLIGHT TO MODERATE WEATHERED 24.8, 24.9 MODERATELY WEATHERED FRACTURE 25.4, 25.8, 25.9, 26.2 - SLIGHT TO MODERATELY WEATHERED FRACTURE 28.1, 29.4, 29.9, 30.3 - SLIGHTLY WEATHERED FRACT 31.1, 31.4 - SLIGHT TO MODERATELY WEATHERED	THERED	RC 7	70 (44)					WOOD TO THE		MARINE CONTRACTOR OF THE CONTR	A THE RESIDENCE OF THE PARTY OF
30		FRACTURE 30.8-31.0 - VERTICAL FRACTURE		RC 8	92 (82)				The state of the s				
		Refusal at 17.0 feet. Bottom of borehole at 32.0 feet.			<u></u>			<u> </u>	<u></u>	<u></u>	<u></u>		

BORING NUMBER E-6

AMEC

11003 Bluegrass Parkway Suite 690 Louisville, KY 40299 502-267-0700

CLIENT Army National Guard	F
PROJECT NUMBER 7-7728-0000-0002	F
DATE STARTED 10/9/08 COMPLETED 10/9/08	(

DRILLING CONTRACTOR MATHES

(Continued Next Page)

DRILLING METHOD HSA LOGGED BY MGS

PROJECT NAME South Gate Road Slope Failure PROJECT LOCATION Camp Dawson, West Virginia GROUND ELEVATION 1271 ft HOLE SIZE 3.25 GROUND WATER LEVELS:

AT TIME OF DRILLING _---

AT END OF DRILLING _-_

LOGGED BY	MGS	T END OF FTER DRI		JNG				······			
NOTES Dry	Hole		T						ERBE	RG	탈
DEPTH (ft) GRAPHIC LOG	MATERIAL DESCRIPTION	SAMPLE TYPE NUMBER	RECOVERY % (RQD)	BLOW COUNTS (N VALUE)	POCKET PEN. (tsf)	DRY UNIT WT. (pcf)	MOISTURE CONTENT (%)		PLASTIC WI	PLASTICITY INDEX	FINES CONTENT (%)
	CLAY, SILTY, LITTLE BLACK NODULES, MIX OF TAN, REDDISH BROWN AND GRAY, STIFF	SS 1		5-5-6 (11)							
5 0	SILT, CLAYEY, TRACE FINE GRAVEL, TRACE ORGANICS, TAN, STIFF	SS 2		5-6-8 (14)							
	CLAY, SILTY, WITH FINE GRAINED SAND, REDDISH BROWN AND GRAY	SH 3				112	19	39	22	17	91
10	CLAY, SILTY, TAN AND REDDISH BROWN, VERY STIFF	SS 4		10-12-15 (27)						A CONTRACTOR OF THE CONTRACTOR	
OPE FAILURE GPJ	SILT, CLAYEY, WITH FINE GRAINED SAND, TAN, STIFF	X 55	3	6-6-6 (12)			15				54
T. 12/7/08 08:45 - N:\cin\tripropects\text{ARIURE.GPJ} Sp	CLAY, SILTY, SOME FINE ROCK FRAGMENTS, MIX OF TAN AND DARK GRAY, STIFF	X s	S	8-7-7 (14)			10)			58
17/08 08:45 - N:/GIP	WEATHERED SHALE, VERY DENSE	 	S 7	46-50/	1*		ANNA ANNA ANNA ANNA ANNA ANNA ANNA ANN	*****			
25 AB.GDT - 12		F	RC 8 8 (7	5 1)							
GEOTECH BH COLUMNS - GINT STD US LAB GD 30 30			3C 9	93 76)		AND THE PERSON NAMED IN COLUMN TWO IS NOT THE OWNER.		***************************************	and the state of t		
35 35 35									-,		

11003 Bluegrass Parkway Suite 690 Louisville, KY 40299 502-267-0700 CLIENT Army National Guard PROJECT NAME PROJECT NAME PROJECT NAME PROJECT LOCATION MATERIAL DESCRIPTION MATERIAL DESCRIPTION MATERIAL DESCRIPTION MATERIAL DESCRIPTION SHALE, LIGHT GRAY SUMMON ON THE MATERIAL DESCRIPTION ATTERBERG LIMITS ATT		AMEC					BO	RIN	G N	IUM	BEI PAGE	RE.	6
PROJECT NUMBER 7-7728-0000-0002 PROJECT NUMBER 7-7728-0000-0002 MATERIAL DESCRIPTION	əmec [©]	11003 Bluegrass Parkway Sulte 690 Louisville, KY 40299									, , , , , , , , , , , , , , , , , , , ,		
PROJECT NUMBER 7-7728-0000-0002 PROJECT LOCATION Camp Dawson, West Virgina MATERIAL DESCRIPTION MATERIAL DES	CLIENT Arr											 	
MATERIAL DESCRIPTION			PROJEC	T LOCATI	ON (Camp Daw	son, V	/est V	irginia	A 49-4-	FDDE		
SHALE, LIGHT GRAY 28.4 - MODERATELY TO HIGHLY WEATHERED FRACTURE 28.9 - SLIGHTLY WEATHERED FRACTURE 29.2.29.4 - MODERATELY WEATHERED FRACTURE 29.8-30.3 - VERTICAL FRACTURE, SLIGHTLY WEATHERED 30.5 - MODERATELY WEATHERED FRACTURE 30.5 - SLIGHTLY WEATHERED FRACTURE 31.8 - SLIGHTLY WEATHERED FRACTURE 33.0 - SLIGHTLY WEATHERED FRACTURE 33.0 - SLIGHTLY WEATHERED FRACTURE 35.2, 36.8-36.9, 37.2, 38.2 - SLIGHTLY WEATHERED FRACTURE 38.9 - MODERATELY WEATHERED FRACTURE 39.3 - SLIGHTLY WEATHERED FRACTURE 39.9 - MODERATELY WEATHERED FRACTURE 40.1, 40.7 - SLIGHTLY WEATHERED Refusal at 25.0 feet. Bottom of borehole at 40.9 feet.	1 1	MATERIAL DESCRIPTION		1	RECOVERY (RQD)	BLOW COUNTS (N VALUE)	POCKET PEN. (tsf)	DRY UNIT WT. (pcf)	MOISTURE CONTENT (%)	l	<u>IMITŞ</u>		FINES CONTENT (%)
NS GINT STD US LAB GDT - 12/17/08 08:45 - N/GINT PHOLECU SQU		28.4 - MODERATELY TO HIGHLY WEATHERED FRACTURE 28.9 - SLIGHTLY WEATHERED FRACTURE 29.2, 29.4 - MODERATELY WEATHERED FRACTURE 29.8-30.3 - VERTICAL FRACTURE, SLIGHTLY WEATHI 30.5 - MODERATELY WEATHERED FRACTURE 30.7 - SLIGHTLY WEATHERED FRACTURE 31.8 - SLIGHTLY WEATHERED FRACTURE 33.0 - SLIGHTLY WEATHERED FRACTURE 33.0 - SLIGHTLY WEATHERED FRACTURE 33.5, 34.9 - MODERATELY WEATHERED FRACTURE 35.2, 36.8-36.9, 37.2, 38.2 - SLIGHTLY WEATHERED FRACTURE 38.9 - MODERATELY WEATHERED FRACTURE 39.3 - SLIGHT TO MODERATELY WEATHERED 40.1, 40.7 - SLIGHTLY WEATHERED FRACTURE (continuo previous page) Refusal at 25.0 feet.	ERED	10	100								
	OCITIMNS - GIN US LYBRED - 123 1798 0832 - 13 18 18 18 18 18 18 18 18 18 18 18 18 18												

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AMEC 11003 Bluegrass Parkway Suite 690 Louisville, KY 40299

502-267-0700	
CLIENT Army National Guard	PROJECT NAME South Gate Road Slope Failure
PROJECT NUMBER 7-7728-0000-0002	PROJECT LOCATION Camp Dawson, West Virginia
DATE STARTED 10/8/08 COMPLETED 10/8/08	GROUND ELEVATION 1266 ft HOLE SIZE 3.25
DRILLING CONTRACTOR MATHES	GROUND WATER LEVELS:
DRILLING METHOD HSA	AT TIME OF DRILLING
LOGGED BY MGS	AT END OF DRILLING AFTER DRILLING
NOTES Dry Hole	
	W % N I I W S LIMITS I

-		T		ш	%		j:	ن ا		AII	IMITS	nu	Z
		GRAPHIC LOG	MATERIAL DESCRIPTION	SAMPLE TYPE NUMBER	RECOVERY ?	BLOW COUNTS (N VALUE)	POCKET PEN. (tsf)	DRY UNIT WT. (pcf)	MOISTURE CONTENT (%)			PLASTICITY INDEX	FINES CONTENT (%)
	0		CLAY, SILTY, SOME FINE TO COARSE SAND, TRACE FINE GRAVEL, BROWN, MEDIUM STIFF	SS 1		3-3-4 (7)							
-	5		CLAY, SILTY, SOME FINE TO COARSE SAND, TRACE FINE GRAVEL, BROWN, MEDIUM STIFF	X SS		3-3-3 (6)							
-			CLAY, SILTY, SOME FINE TO COARSE SAND, BROWN, VERY STIFF	SS 3	1	8-6-10 (16)			15				53
	10		CLAY, SILTY, SOME FINE TO COARSE SAND, BROWN, STIFF	SS 4		4-7-8 (15)			15				65
4G SLOPE FAILURE.GPJ	15	00000	SILT, CLAYEY, TRACE FINE TO COARSE ROCK FRAGMENTS, BROWN, HARD	X SS 5		6-8-28 (36)			AND THE PROPERTY OF THE PROPER	The state of the s	in the state of th		
NTAPROJECTSVAR	20		SILT, CLAYEY, DARK GRAY, STIFF	X SS	3	6-8-7 (15)			15				53
GENTECH BH COLLIMMS - GINT STD US LAB GDT - 12/17/08 08:45 - NAGINTAPROJECTSVARING SLOPE FAILURE.GPJ			Refusal at 21.0 feet. Bottom of borehole at 21.0 feet.										
HE HUELUED													

,	∧	AMEC 11003 Bluegrass Parkway Suite 690			, , , , , , , , , , , , , , , , , , , 		во	RIN	G N			R E	
am		Louisville, KY 40299 502-267-0700											
CLIEN	T Am	ny National Guard	PROJEC	_		Gate Road							
PROJ	ECT N	JMBER _7-7728-0000-0002	PROJEC	T LOCAT	ON _	Camp Daw	son, W	est Vi	rginia				
		TED 10/8/08 COMPLETED 10/8/08						HOLE	SIZE	3.25			[
DRILL	ING C	ONTRACTOR MATHES	GROUND	WATER	LEVE	LS:							
DRILL	JNG M	ETHOD HSA				JNG							
LOGG	ED BY	MGS				ING							
NOTE	S Dry	Hole	AF	TER DAII	LING					<u> </u>	ERBE	DC	
E	SE @			TYPE	ERY % ID)	NW NATS LUE)	T PEN.	UT WT.	MOISTURE CONTENT (%)	<u> </u>	IMITS		ONTEN
DEPTH (ft)	GRAPHIC LOG	MATERIAL DESCRIPTION		SAMPLE TYPE NUMBER	RECOVERY '	BLOW COUNTS (N VALUE)	POCKET PEN. (tsf)	DRY UNIT WT. (pcf)	MOIS	LIMIT	PLAST LIMIT	PLASTICITY INDEX	FINES CONTENT (%)
-	1717	CLAY, SILTY, LITTLE FINE TO COARSE GRAVEL, RE	DDISH										
-		BROWN AND GRAY, VERY STIFF		SS 1		8-9-11 (20)							
5		CLAY, SILTY, TRACE FINE GRAVEL, TAN, STIFF		SS 2		9-5-7 (12)	-						
-		CLAY, SILTY, LITTLE ORGANICS, LITTLE FINE TO CO SAND, TRACE FINE GRAVEL, MIX OF REDDISH BROV GRAY, STIFF	ARSE WN AND	X ss 3		3-6-6 (12)							
10		CLAY, SILTY, LITTLE FINE TO COARSE SAND, LITTLE NODULES, MIX OF GRAY AND BROWN, STIFF	BLACK	SS 4		3-5-5 (10)				THE STATE OF THE S			
15		CLAY, SILTY, SOME FINE TO COARSE ROCK FRAGM LITTLE BLACK NODULES, BROWN AND REDDISH BF MEDIUM STIFF	IENTS, ROWN,	SS 5		4-3-3 (6)	W. (1)	ANALYSIS OF THE PROPERTY OF TH					
2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	-	SILT, CLAYEY, BROWN AND REDDISH BROWN, STIF	F	X ss 6		3-4-6 (10)			16				
20				V V 0		(10)				_		The state of the s	
3		SHALE		1									
<u> </u>		Refusal at 23.5 feet.	·····	SS 7	 	50/1"	J					<u> </u>	
3		Bottom of borehole at 23.5 feet.		<u> 7</u>	J		-						
2													
5													
2													
2													
- Cells													
LOBBIN													
S S S S S S S S S S S S S S S S S S S	t dispetitive exp	rang andre en andre descript affects — Affects of all the first tenderal control of the control	tour and the strength or decommends with	wike frequency benquipays or pros	enggraf, egines, af ha	e' er op he' erere, e with Jy er it efuelen anbe	udati matud yaz an-tu	m ki dhesakari k	nor. 5 - 2 - 2 - 2 - 2 - 2 - 2 - 2 - 2 - 2 -		~~~		
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11003 Bluegrass Parkway Suite 690 Louisville, KY 40299 502-267-0700	PAGE 1 OF 2
CLIENT Army National Guard	PROJECT NAME South Gate Road Slope Failure
PROJECT NUMBER 7-7728-0000-0002	PROJECT LOCATION Camp Dawson, West Virginia
DATE STARTED 10/8/08 COMPLETED 10/8/08	GROUND ELEVATION 1266.6 ft HOLE SIZE 3.25
DRILLING CONTRACTOR MATHES	GROUND WATER LEVELS:
DRILLING METHOD HSA	AT TIME OF DRILLING
OGGED BY MGS	AT END OF DRILLING
NOTES Dry Hole	AFTER DRILLING
	ATTERBERG LIMITS LIMITS

LOGGED B	Y MGS	AT END	OF DF	3ILLI	NG <u></u>								
NOTES D	ry Hole												
		Y PE	r >	9	တ 🗓	EN.	WT.	RΕ (%)		IMITS	}	TENT	
O DEPTH (ft) GRAPHIC		SAMPLE TYPE	NUMBE	(RQD)	BLOW COUNTS (N VALUE)	POCKET PEN. (ISI)	DRY UNIT WT. (paf)	MOISTURE CONTENT (%)	LIQUID	PLASTIC LIMIT	PLASTICITY INDEX	FINES CONTENT (%)	
	SILT, CLAYEY, WITH FINE TO COARSE GRAVEL, SOME BLACK NODULE, BROWN, VERY STIFF		38		6-9-14								
	d	__	1		(23)								
5	SAND, SILTY, TRACE FINE TO COARSE SAND, TRACE FINE GRAVEL, TRACE ORGANICS, BROWN, STIFF		SS 2		6-6-8 (14)			16				47	
	CLAY, SILTY, LITTLE FINE TO COARSE SAND, TRACE BLACK NODULE, GRAY, MEDIUM STIFF	X	8S 3		4-2-3 (5)			15	38	23	15		
10	SILT, CLAYEY, TRACE FINE GRAVEL, BROWN, MEDIUM STIFF	M	SS 4		3-3-4 (7)						· Dayson - D		
s stope FAILURE GPU	CLAY, SILTY, SOME FINE TO COARSE SAND, LITTLE BLAC NODULES, BROWN, STIFF	<u></u>	SS 5		2-4-6 (10)				THE PROPERTY OF THE PROPERTY O			· · · · · · · · · · · · · · · · · · ·	
NYGINTY-ROJECTSVARING	SILT, CLAYEY, LITTLE WEATHERED SHALE, BROWN AND TAN, VERY STIFF		SS 6		6-8-10 (18)			. Language de la constant	The state of the s				
GEOTECH BH COLUMNIS - GINT STD US LAB, GDT - 12/17/08 08:46 - NAGINTI-PROJECTSVARNG SLOPE FAILURE, GPJ GEOTECH BH COLUMNIS - GINT STD US LAB, GDT - 12/17/08 08:46 - NAGINTI-PROJECTSVARNG SLOPE FAILURE, GPJ GEOTECH BH COLUMNIS - GINT STD US LAB, GDT - 12/17/08 08:46 - NAGINTI-PROJECTSVARNG SLOPE FAILURE, GPJ GEOTECH BH COLUMNIS - GINT STD US LAB, GDT - 12/17/08 08:46 - NAGINTI-PROJECTSVARNG SLOPE FAILURE, GPJ GEOTECH BH COLUMNIS - GINT STD US LAB, GDT - 12/17/08 08:46 - NAGINTI-PROJECTSVARNG SLOPE FAILURE, GPJ GEOTECH BH COLUMNIS - GINT STD US LAB, GDT - 12/17/08 08:46 - NAGINTI-PROJECTSVARNG SLOPE FAILURE, GPJ GEOTECH BH COLUMNIS - GINT STD US LAB, GDT - 12/17/08 08:46 - NAGINTI-PROJECTSVARNG SLOPE FAILURE, GPJ GEOTECH BH COLUMNIS - GINT STD US LAB, GDT - 12/17/08 08:46 - NAGINTI-PROJECTSVARNG SLOPE FAILURE, GPJ GEOTECH BH COLUMNIS - GINT STD US LAB, GDT - 12/17/08 08:46 - NAGINTI-PROJECTSVARNG SLOPE FAILURE, GPJ GEOTECH BH COLUMNIS - GINT STD US LAB, GDT - 12/17/08 08:46 - NAGINTI-PROJECTSVARNG SLOPE FAILURE, GPJ GEOTECH BH COLUMNIS - GINT STD US LAB, GDT - 12/17/08 08:46 - NAGINT STD US LAB, G	SILT, CLAYEY, WITH FINE GRAINED SAND, REDDISH BROWN, MEDIUM STIFF	 	SS 7		3-2-4 (6)				esternature de la biologia de la companya de la com		The state of the s		
JUMNS - GINT STD US			RC 8	97 (20)				***************************************	And the second s				
TO HER	The state of the s		RC 9	91 (51)	+		-						
35	ACCURATE AND ACCUR												
-	(Continued Next Page)												

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11003 Bluegrass Parkway Suite 690 Louisville, KY 40299 502-267-0700

CLIENT Army National Guard

PROJECT NUMBER <u>7-7728-0000-0002</u>

PROJECT NAME South Gate Road Slope Failure

PROJECT LOCATION Camp Dawson, West Virginia

BORING NUMBER E-9

PAGE 2 OF 2,

FINES CONTENT (%)

<u> </u>			Ļ	Ų	%		ż	WT.	三 (%)		ERBE		
DEPTH (II)	GRAPHIC LOG	MATERIAL DESCRIPTION	7	NUMBER	RECOVERY (RQD)	BLOW COUNTS (N VALUE)	POCKET PEN. (tsf)	DRY UNIT W (pct)	T.	LIQUID	PLASTIC LIMIT	PLASTICITY INDEX	
"		SHALE, LIGHT GRAY	Н						ļ				
40		27.95, 28.15, 28.5, 29, 29.1, 29.3, 29.5 - MODERATE TO HIGHLY WEATHERED FRACTURE 29.7, 29.8, 30.0 - SLIGHT TO MODERATELY WEATHERED FRACTURE 30.1, 30.2, 30.4 - HIGHLY WEATHERED FRACTURE 31.1, 31.4 - SLIGHTLY WEATHERED FRACTURE 31.6, 31.7 - MODERATELY WEATHERED FRACTURE		RC 10	100 (54)		A TOTAL AND THE STREET, THE ST		The state of the s		A A A A A A A A A A A A A A A A A A A		The second secon
		31.9, 32.1 - HIGHLY WEATHERED FRACTURE 32.7 - SLIGHTLY WEATHERED FRACTURE 33.4-33.5 - HIGHLY WEATHERED ZONE OF LOSS		RC 11	100 (50)								
	The state of the s	3.9, 34.0 - MODERATELY WEATHERED FRACTURE 34.5 - SLIGHTLY WEATHERED FRACTURE 34.7 - MODERATELY WEATHERED FRACTURE 36.5 - SLIGHTLY WEATHERED FRACTURE 38.9, 37.6, 38.1, 38.3, 38.4, 38.6 - MODERATELY WEATHERED FRACTURE 39, 39.3, 39.4, 39.5, 39.6, 39.8 - SLIGHT TO MODERATELY WEATHERED											The state of the s

40.7, 40.9 -41.0 - MODERATELY WEATHERED FRACTURE 41.5, 41.7, 42.0 - MODERATE TO HIGHLY WEATHERED FRACTURE (continued from previous page) Refusal at 27.5 feet. Bottom of borehole at 42.5 feet.

GEOTECH BH COLUMNS - GINT STD US LAB.GDT - 12/17/08 08:46 - NY.GINTY-PROJECTSVARING SLOPE FAILURE.GPJ

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BORING NUMBER E-10

MK	LO	AMEC 11003 Bluegrass Parkway Suite 690 Louisville, KY 40299 502-267-0700								F	AGE	1 OF	2
~! ! !	r Associ		PROJECT	NAME	South	Gate Road	Slope	Failu	re		·-····		
000 IE	CT NH	9 National Codio	PROJECT	LOCATI	ON C	amp Daws	on, We	st Vii	rginia		·····		
DATE:	START	ED 10/7/08 COMPLETED 10/7/08	GROUND	ELEVAT	ION <u>1</u>	268.8 ft	н	OLE S	SIZE ,	3.25			
וומו	NG CO	NTRACTOR MATHES	GROUND	WATER	LEVEL	S:							1
		THOD HSA	AT.	TIME OF	DRILL	NG							
		MGS	AT	END OF	DRILLI	NG							
	S Dry		AF	TER DRII	LING .								
1				ш	%		·	<u>.</u>	્	ATT	ERBE	RG	
DEPTH	GRAPHIC LOG	MATERIAL DESCRIPTION		SAMPLE TYPE NUMBER	RECOVERY (RQD)	BLOW COUNTS (N VALUE)	POCKET PEN. (tsf)	DRY UNIT WT. (pcf)	MOISTURE CONTENT (%)	LIMIT	PLASTIC LIMIT	PLASTICITY INDEX	FINES CONTENT (%)
0	77X	GRAVEL, FINE TO COARSE, SOME SILTY CLAY, MED	IUM										
		DENSE		X ss 1		6-6-5 (11)							
 5		CLAY, SILTY, SOME FINE TO COARSE SAND, BROWN MEDIUM STIFF	Ν,	X SS 2		2-3-5 (8)							
		CLAY, SILTY, SOME FINE TO COARSE SAND, BROWN TAN, SOFT	N AND -	SS 3		1-2-2 (4)							
10		CLAY, SILTY, SOME FINE TO MEDIUM GRAINED SAN BROWN AND GRAY, SOFT	īD,	SS 4		1-1-2 (3)		**************************************				1,000	
-		CLAY, SILTY, LITTLE FINE TO COARSE SAND, BROW	VN, STIFF	SS 5		2-4-5 (9)			**************************************				
15		CLAY, SILTY, SOME FINE TO COARSE SAND, SOME	FINE TO	<u> </u>		(0)							
20		COARSE ROCK FRAGMENTS, BHOWN, HAND			5	4-50/5*	_						
-	4	SANDSTONE, HIGHLY WEATHERED											
25		SHALE, GRAY, HIGHLY WEATHERED, SOME SILTY	CLAY					***************************************			A STATE OF THE STA		
							The state of the s				V		
SECTECH BH: COLUMNS - GINT STD US LAB. GD.				s	S 100 (38						And the second s		
REOTECH BH CO				\\\s	S 94								

(Continued Next Page)

GRAPHIC

DEPTH (ft)

GEOTECH BH COLLMANS - GINT STD US LAB GDT - 12/17/08 08:34 - NAGINTPROJECTSVARNG SLOPE FAILURE.GPJ

BORING NUMBER E-10

DRY UNIT WT. (pcf)

MOISTURE CONTENT (%)

POCKET PEN. (tsf)

PAGE 2 OF 2.

PLASTICITY SHIPPEX BLANDEX FINES CONTENT (%)

ATTERBERG LIMITS

LIQUID LIMIT PLASTIC LIMIT

AMEC			
11003 Blueg	rass Parkway	Suite	690
Louisville, K	40299		

502-267-0700

PROJECT NAME South Gate Road Slope Failure CLIENT Army National Guard

PROJECT LOCATION Camp Dawson, West Virginia PROJECT NUMBER 7-7728-0000-0002

SAMPLE TYPE NUMBER

RECOVERY % (RQD)

100 (100) SS

•••
SHALE, LIGHT GRAY
27.4- SLIGHTLY WEATHERED FRACTURE 27.5 - MODERATE TO HIGHLY WEATHERED FRACTURE 27.8 - SLIGHTLY WEATHERED FRACTURE 27.9 - HIGHLY WEATHERED, MUD SEAM 28.1 - HIGHLY WEATHERED, MUD SEAM 28.4-28.6 - HIGHLY WEATHERED, MUD SEAM 28.9, 29.0 - MODERATE TO HIGHLY WEATHERED FRACTURE 29.2 - SLIGHTLY WEATHERED FRACTURE 29.4, 29.9 - MODERATE TO HIGHLY WEATHERED FRACTURE 30.2, 30.4 - SLIGHTLY WEATHERED FRACTURE 30.7 - SLIGHT TO MODERATELY WEATHERED FRACTURE 30.7-30.9 - SLIGHTLY WEATHERED FRACTURE 31.0, 31.1, 31.4, 31.6 - HIGHLY WEATHERED, MUD SEAM 32.2, 32.4, 32.7 - MODERATELY WEATHERED FRACTURE 33.0, 33.1, 33.3 - SLIGHTLY WEATHERED FRACTURE 33.5, 33.8, 33.9, 34.1, 34.4, 34.5, 34.6- MODERATELY WEATHERED FRACTURE 34.9 - SLIGHTLY WEATHERED FRACTURE 35.44, 35.7, 35.8 -MODERATELY WEATHERED FRACTURE (continued from previous page)
Refusal at 27.0 feet. Bottom of borehole at 39.0 feet.
DUMUNI OI DOICHOIS AL 33.0 ISSL.

MATERIAL DESCRIPTION

AMEC

am	*	11003 Bluegrass Parkway Suite 690 Louisville, KY 40299 502-267-0700											
CLIEN	T Arm	Trustoria Guara	ROJECT N										
PROJ	ECT NU		ROJECT L										
DATE	START	ED 10/7/08 COMPLETED 10/7/08 GI	ROUND EL	EVAT	ION	1267.9 ft	1	HOLE	SIZE	3.25		·······	
DRILL	ING CO	NTRACTOR MATHES G	ROUND W	ATER	LEVE	.S:							
		THOD HSA	AT TI	AE OF	DRILL	JNG							
LOGG	ED BY	MGS	AT EN	D OF	DRILL	ING	<u> </u>						······
NOTE	S Dry	Hole	AFTE	R DRII	LING								
			1	ប	8		ż	 	⊊	ATI	ERBE IMITS	HG	Z
DEPTH (ff)	GRAPHIC LOG	MATERIAL DESCRIPTION		SAMPLE I THE NUMBER	RECOVERY (RAD)	BLOW COUNTS (N VALUE)	POCKET PEN. (tsf)	DRY UNIT W	MOISTURE CONTENT (%)	LIQUID	PLASTIC LIMIT	PLASTICITY INDEX	FINES CONTENT
		CLAY, SILTY, LITTLE FINE TO MEDIUM GRAINED SAND, BROWN, MEDIUM STIFF (PROBABLE FILL)	X	SS 1		5-4-3 (7)							
5		CLAY, SILTY, WITH FINE TO COARSE GRAINED SAND, AND BROWN, MEDIUM STIFF (PROBABLE FILL)	X	SS 2		3-3-4 (7)	-						
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10		CLAY, SILTY, LITTLE FINE TO COARSE SAND, SOME ORGANICS, ORGANIC ODOR, DARK GRAY, SOFT		SS 4		2-2-2 (4)		Topic and the second se			And the second s	THE PERSON NAMED IN COLUMN NAM	
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BORING NUMBER E-12

am	ec®	AMEC 11003 Bluegrass Parkway Suite 690 Louisville, KY 40299 502-267-0700								i	PAGE	1 OF	: 1
CHEN	CLIENT Army National Guard PROJECT NAME South Gate Road Slope Failure												
1	PROJECT NUMBER 7-7728-0000-0002 PROJECT LOCATION Camp Dawson, West Virginia												
DATE STARTED 10/7/08 COMPLETED 10/7/08 GROUND ELEVATION 1267 ft HOLE SIZE 3.25													
DRILLING CONTRACTOR MATHES GROUND WATER LEVELS:													
DRILLING METHOD HSA AT TIME OF DRILLING													
LOGGED BY MGS AT END OF DRILLING													
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10		NO RECOVERY		SS 4		8-4-3 (7)				the statement of the st			
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I STD US LAB.GDI		Refusal at 28.2 feet.								- Lander Linear			
Bottom of borehole at 28.2 feet.													
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614.3

Y	MILS (mm)			
В	79 (2.0 mm)			
С	C 109 (2.7 mm)			
D	138 (3.5 mm)			
Е	168 (4.3 mm)			
F	188 (4.8 mm)			
G	218 (5.5 mm)			
Н	249 (6.3 mm)			
J	280 (7.1 mm)			

613-BLANK

SECTION 614 PILING WALLS

614.1 - DESCRIPTION:

This work shall consist of furnishing and placing steel piles in predrilled holes, concrete or grout, backfill and lagging, of the kinds and dimensions designated, in accordance with these provisions and in reasonably close conformity with the lines, grades, dimensions, and locations shown on the Plans or established by the Engineer. Painting of the exposed steel is included.

Careful attention shall be given to assuring the pile wall will tie directly into an existing stable slope. Prior to ordering any materials, the contractor in conjunction with the Engineer shall conduct a project site review in order to verify the limits of the pile wall.

614,2 - MATERIALS:

Materials shall conform to the requirements specified in the following Subsections of Division 700:

MATERIAL	SUBSECTION
Steel Piles and Splices	709.12
Steel Lagging and Wales	709.12
Reinforcing Steel	709.1
Prestressing Steel	709.2
Treated Timber Lagging	710
Portland Cement	701.1
Fine Aggregate	702.1
Fly Ash	707.4

614.3 - DRILLING:

614.4

A drilled hole is required for the buried length of the pile.

A minimum of 1/3 the total pile length or 10 feet (3 m), whichever is greater, is to be placed in bedrock/shale. Deviation from this requirement will be controlled by a Plan note. The total estimated pile length and the depth to the estimated bedrock/shale line are shown on the piling profile. Should the elevation of the actual bedrock/shale vary from the estimated elevation by more than 2.5 feet (0.8 m), the Engineer must approve the hole prior to placement of the pile. The material from the drilled hole shall be removed and disposed of by the Contractor in an approved site.

Particular care must be taken in the drilling operation to avoid deflecting the bit along a sloping bedrock/shale line. To verify proper alignment, the Contractor shall measure and record the vertical alignment of the hole using a plumb bob or other acceptable method.

Preferably, the diameter of the drilled hole shall be a size that will allow the pile, while being slowly lowered into the hole, to reach the bottom of the hole under the impetus of the pile weight. The minimum hole diameter shall be 2 inches (50 mm) larger than the diagonal distance across the pile cross section.

Light tapping (ten blows with at least 3 inches (75 mm) of penetration per blow) with a pile hammer exerting no more than 12,000 ft/lbs (16 kJ) of energy is permitted at the direction of the Engineer to advance the pile past minor obstacles in the hole.

Temporary casing of holes may be needed to maintain an open clean hole through the soil overburden. There will be no additional compensation for temporary casing. The cost of any casing used shall be included in the unit price bid for piling.

614.4 - INSTALLATION OF PILES:

Piles shall be located as shown on the Plans or as directed by the Engineer. Piles shall be installed with the pile center within 1 inch (25 mm) of the Plan location. The piles must be prevented from rotating, so that the pile axis is within five degrees of the position shown on the Plans.

The maximum permissible vertical deviation for piles shall be one percent of the total pile length, as measured at the actual pile location.

It is desirable that piles be installed without splicing; however, at the direction of the Engineer splices may be made. Splice lengths at the top of the piles may be butt welded provided the splice lengths are less than the required splice plates. No payment will be made for cut-offs. Welding shall be in accordance with 615.3.16.

Accurate records shall be maintained by the Contractor showing the depth to which each pile was placed, the plumbness, the amount of material used, elevation of bedrock/shale, and any unusual conditions encountered during the pile installation. These records shall be incorporated into the permanent records of the project.

614.5 - CORROSION PROTECTION:

Piles will be protected from corrosion and sealed by the placement of

concrete or grout, from the bottom of the hole to the bottom of the lagging or as directed by the Engineer. Vibration of the concrete or grout is not required. The Contractor shall complete all concrete or grout operations for holes drilled during the work day.

The drilled hole shall be pumped free of water and shall be reasonably free of fall-in soil or other debris prior to the placement of the concrete or grout. The concrete or grout in the bedrock/shale portion of the hole will be pumped or tremied through a pipe beginning at the bottom of the drilled hole. The pipe shall be slowly raised ensuring the pipe end remains at least 2 feet (600 mm) below the surface of the concrete or grout. A means of positively measuring the elevation of the concrete or grout as it is placed shall be provided by the Contractor.

After placing the concrete or grout in the bedrock/shale, the Contractor has the option of either pumping or pouring directly into the hole the remainder of the concrete or grout. Placing the concrete or grout from the bottom of the hole to the bottom of the lagging shall be accomplished in one continuous operation.

The Contractor will inform the Engineer, at the preconstruction conference, as to the type of corrosion protection that will be used. Intermixing of concrete and grout will not be allowed, unless approved by the Engineer.

Concrete shall be in accordance with Section 601, Class B. The job site testing is waived

Grout will be furnished and placed in accordance with the requirements specified herein.

The acceptance sampling and testing of the grout is the responsibility of the Division.

Quality Control of the concrete or grout is there responsibility of the Contractor as designated in Materials Procedure MP 601.03.50. The Contractor shall maintain equipment and qualified personnel, who shall direct all field inspection, sampling, and testing necessary to determine the magnitude of the various properties of the concrete and grout governed by the Specifications and shall maintain these properties within the limits of this Specification. The Quality Control Plan designated in MP 601.03.50 shall be submitted to the Engineer at the pre construction conference. Work shall not begin until the Plan is reviewed for conformance with the contract documents.

The required 7-day compression strength of the grout shall be a minimum of 2,000 psi (14 MPa). Grout which does not attain the 2,000 psi (14 MPa) strength in 7 days but exceeds a strength of 1,600 psi (11 MPa) shall be subject to price reduction based on the percentage of strength attained.

A grout strength test shall consist of testing three 6 in x 12 in (150 mm x 300 mm) cylindrical specimens. The test results shall be the average of the three specimens. One set of three specimens shall be made for each day's operations.

The bid price for the piling with grout compressive strengths greater than or equal to 2,000 psi (14 MPa) will be paid at 100 percent unless the piling installation does not meet Specifications for other reasons. Between 1,600 psi

614.6

(11 MPa) and 2,000 psi (14 MPa) compressive strengths, the cost of the grout will be deducted from the actual grout cost on a proportional basis with 2,000 psi (14 MPa) being 100 percent and 1,600 psi (11 MPa) being zero percent payment. With 1,600 psi (11 MPa) grout, the piling installation would be considered to meet 80 percent of the Specifications and the penalty being zero payment for the grout.

The penalty would involve only the quantity of grout represented by the failing compressive strength results.

The bid price for the piling will be reduced for the piles grouted with grout having less than 1,600 psi (11 MPa) compressive strengths as follows:

A = Compressive strength of grout

B = Total foot (meter) of piling grouted with

C = Unit bid price per foot (meter) of piling

D = Cost of grout (from Contractor)

E = 2,000 psi (14 Mpa)

F = Total penalty

F = D + [BC - D] 10.80 - (A + E)

614.6 - PAINTING:

All surfaces from the top of the steel pile, down to and including 2.0 ft. (600 mm) below the top of the anticipated grout line shall be cleaned and painted. The method of surface preparation shall be hand tool cleaning to SSPC-SP-2. The paint system shall consist of one-coat of aluminum epoxy mastic meeting the requirements of 711.12 applied at a minimum dry film thickness of 5 mils ($125 \mu m$).

614.7 - LAGGING AND BACKFILLING:

Lagging of the type and size as specified on the Plans shall be installed between the piles. Backfilling and restoration of the roadway template shall be as shown on the Plans.

Timber lagging shall be Grade #3 or better treated rough cut oak, 3 in (75 mm) wide by 8 in (200 mm) deep for heights up to 11 ft (3.4 m); and for wall heights exceeding 11 ft (3.4 m) the timber lagging shall be double 3 in (75 mm) wide by 8 in (200 mm) deep. The boards shall be cut to their required length prior to preservative treatment.

The timber lagging shall conform to Sections 710.3 and 710.4 of the West Virginia Division of Highways Standard Specifications and shall be CCA treated for soil and fresh water use, as per AWPA C2.

614.8 - METHOD OF MEASUREMENT:

The quantity of piles will be measured in linear feet (meters) of piles installed and accepted for the wall. The quantity of lagging will be measured in square feet (meters) of lagging installed and accepted for the wall.

614.9-BASIS OF PAYMENT:

The quantities will be paid for at the contract unit prices bid for the items listed below, which prices and payments shall be full compensation for furnishing all materials and doing all the work herein prescribed in a workmanlike and acceptable manner, including all labor, tools, equipment, supplies, and incidentals necessary to complete the work. The cost of drilling, concrete, grout, wales, and painting shall be included in the price bid for the piles. The cost of painting and welding steel lagging shall be included in the price bid for steel lagging.

614.10-PAY ITEMS:

ITEM	DESCRIPTION	UNIT
614001-*	"size" STEEL PILE	LINEAR FOOT (METER)
614002-*	STEEL LAGGING, THICKNESS "thickness"	SQUARE FOOT (METER)
614003-*	CONCRETE LAGGING, THICKNESS "thickness"	SQUARE FOOT (METER)
614004-*	TIMBER LAGGING	SQUARE FOOT (METER)

^{*} Sequence number



SUPPLEMENTARY SPECIFICATIONS

The specifications for project shall be the West Virginia Department of Highways, "Standard Specifications, Roadways and Bridges", except as modified herein.

614.1 - DESCRIPTION

Delete the last sentence in the first paragraph

614.3 - DRILLING

Revise the second paragraph to read the following:

"The minimum embedment of the pile length into bedrock shall be designated on the plans. The total estimated pile length and depth to estimated bedrock are shown in the 'Drilled Shaft Schedule' and on the 'Drilled Shaft Profile'. Should the actual elevation vary by more than 2.5 feet, the Engineer must approve the hole prior to placement of the pile. The material from the drilled hole shall be removed and disposed of by the Contractor in an approved site."

Revise portions of the fourth paragraph to state the minimum diameter of the drilled hole shall be as shown on the plans.

614.5 - CORROSION PROTECTION

Revise the first paragraph to indicate that vibration of concrete will be required for the upper ten feet of the drilled shaft.

Delete from paragraph 11 to the end of section 614.5 inclusively. All concrete is expected to reach a minimum 7 day strength of 1,600 psi and 28 day strength of 4,000 psi.

614.6 - PAINTING

Delete this section in its entirety. Painting of the structural steel is not required.

614.7 - LAGGING AND BACKFILLING

Delete paragraphs two and three in this section. Timber lagging is not applicable to this project.

MEASUREMENT AND PAYMENT

LINE ITEM 1 - Mobilization and Demobilization

This item will cover the payment for the mobilization and demobilization of all plant and equipment to execute the project. Payment will be made on a LUMP SUM basis.

LINE ITEM 2 – Clear & Grub

This item will cover the payment for clearing and grubbing the area of work, of all trees, shrubs, etc. Payment will be made on a PER ACRE basis.

LINE ITEM 3 – Excavation and Embankment

This item will cover payment for all excavation and embankment not otherwise included in any other line items on the Bid Form. These items include, but are not limited to, excavation and grading in front of the drilled shaft wall, excavation for the precast concrete lagging, any and all benching that may be required, miscellaneous backfill that may be required, etc. Payment will be made on a LUMP SUM basis.

LINE ITEM 4 – Erosion & Sediment Control

This item will cover all erosion and sediment control measures incorporated by the Contractor's approved SWPPP. Payment will be made on a LUMP SUM basis.

LINE ITEM 5 - Roadway Grading

This item will cover the scarifiying, mixing and recompacting of the upper 12" of the existing roadbed. Payment for this item will be made on a per CUBIC YARD basis.

LINE ITEM 6 – Stone Base for Road

This item will cover the 12" of ABC crushed stone to be placed for the new roadway. Payment for this item will be made on a per TON basis.

LINE ITEM 7 – 30" Diameter Drilled Shafts, Above Bedrock

This item will cover all means, methods and materials to perform the machine excavation of the material above bedrock at the drilled shaft locations. Also included in this item shall be the concrete fill material, spoil removal, and forming above grade if necessary. Payment for this item will be made on a per LINEAR FOOT basis.

LINE ITEM 8 – 30" Diameter Drilled Shafts, Into Bedrock

This item will cover all means, methods and materials to perform the machine excavation of the material into bedrock at the drilled shaft locations. Also included in this item shall be the concrete fill material and spoil removal. Payment for this item will be made on a per LINEAR FOOT basis.

LINE ITEM 9 – Steel Piles, W18x106

This item will cover all means, methods and materials to furnish, fabricate and place the steel piles at the proper centerline locations indicated. Payment for this item will be made on a per LINEAR FOOT basis.

LINE ITEM 10 - Steel Piles, W21x111

This item will cover all means, methods and materials to furnish, fabricate and place the steel piles at the proper centerline locations indicated. Payment for this item will be made on a per LINEAR FOOT basis.

LINE ITEM 11 – Precast Concrete Lagging, 8" Thick

This item will cover all means, methods and materials to furnish, fabricate and place the 8" thick precast concrete lagging at the proper locations indicated. Payment for this item will be made on a per SQUARE FOOT basis.

LINE ITEM 12 – Free Draining Backfill

This item will cover all excavation, furnishing & installation of backfill, compaction necessary and furnishing & installation filter fabric. Payment for this item will be made on a per TON basis.

LINE ITEM 13 – 6" Perforated Pipe

This item will cover furnishing & installation of the 6" perforated pipe embedded in the free draining backfill. Payment for this item will be made on a per LINEAR FOOT basis.

LINE ITEM 14 – Cable Guardrail

This item will cover all means, methods and materials to furnish, fabricate and place the cable guardrail at the proper locations indicated. Payment for this item will be made on a per LINEAR FOOT basis.

LINE ITEM 15 - Seed

This item will cover all means, methods and materials to furnish and place the seeding over all areas disturbed. Payment for this item will be made on a per ACRE basis.

		Failure
		Slope
		Road
	RNG	n Gate
••	WVA	South

Bid Item Number	Description	Quantity	Unit	Unit Price	Total Price
-	Mobilization and Demobilization	1.00	เรา		ا ده
Ŋ	Clear & Grub	0.25	ACRE		9
3	Excavation and Embankment	1.00	ട്ട		<u>.</u>
4	Erosion & Sediment Control	1.00	rs LS		٠ ن
5	Roadway Grading	136.00	≿		-
9	Stone Base for Road	272.00	TON		· У
7	30" Diameter Drilled Shafts, Above Bedrock	481.57	占		9
ထ	30" Diameter Drilled Shafts, Into Bedrock	360.00	크		&
6	Steel Piles, W18x106	680.00	띸	****	٠ ده
10	Steel Piles, W21x111	200.00	占		· У
-	Precast Concrete Lagging, 8" Thick	2,070.00	SFT		, &
12	Free Draining Backfill	150.00	TON		\$
ل 13	6" Perforated Pipe	230.00	트		- ھ
4	Cable Guardrail	280.00	坘		-
ट	Seed	0.50	0.50 ACRE		
		T	TOTAL BID	0	٠ د

Contractor Name:Address:	Date:	Signed:



State of West Virginia DRUG FREE WORKPLACE CONFORMANCE AFFIDAVIT West Virginia Code §21-1D-5

STA	TE OF		
COL	INTY OF		, TO-WIT:
I,			, after being first duly sworn, depose and
stat	e as follows	a •	
1.	I am an e	employee of	(Company Name)
2.	I do here	by attest that _	(Company Name)
	maintain policy is	s a valid written in compliance w	drug free workplace policy and that such ith West Virginia Code §21-1D-5.
The	above state	ements are swoi	rn to under the penalty of perjury.
			(Company Name)
			Ву:
			Title:
			Date:
Tak	cen, subscri	bed and sworn t	o before me this day of
Ву	Commission	expires	
(Se	eal)		
			(Notary Public)

THIS AFFIDAVIT MUST BE SUBMITTED WITH THE BID IN ORDER TO COMPLY WITH WV CODE PROVISIONS. FAILURE TO INCLUDE THE AFFIDAVIT WITH THE BID SHALL RESULT IN DISQUALIFICATION OF THE BID.

Rev March 2009

RFC	No.	DEFK9020	43

STATE OF WEST VIRGINIA Purchasing Division

PURCHASING AFFIDAVIT

VENDOR OWING A DEBT TO THE STATE:

West Virginia Code §5A-3-10a provides that: No contract or renewal of any contract may be awarded by the state or any of its political subdivisions to any vendor or prospective vendor when the vendor or prospective vendor or a related party to the vendor or prospective vendor is a debtor and the debt owed is an amount greater than one thousand dollars in the aggregate.

PUBLIC IMPROVEMENT CONTRACTS & DRUG-FREE WORKPLACE ACT:

If this is a solicitation for a public improvement construction contract, the vendor, by its signature below, affirms that it has a written plan for a drug-free workplace policy in compliance with Article 1D, Chapter 21 of the **West Virginia Code**. The vendor **must** make said affirmation with its bid submission. Further, public improvement construction contract may not be awarded to a vendor who does not have a written plan for a drug-free workplace policy in compliance with Article 1D, Chapter 21 of the **West Virginia Code** and who has not submitted that plan to the appropriate contracting authority in timely fashion. For a vendor who is a subcontractor, compliance with Section 5, Article 1D, Chapter 21 of the **West Virginia Code** may take place before their work on the public improvement is begun.

ANTITRUST:

In submitting a bid to any agency for the state of West Virginia, the bidder offers and agrees that if the bid is accepted the bidder will convey, sell, assign or transfer to the state of West Virginia all rights, title and interest in and to all causes of action it may now or hereafter acquire under the antitrust laws of the United States and the state of West Virginia for price fixing and/or unreasonable restraints of trade relating to the particular commodities or services purchased or acquired by the state of West Virginia. Such assignment shall be made and become effective at the time the purchasing agency tenders the initial payment to the bidder.

I certify that this bid is made without prior understanding, agreement, or connection with any corporation, firm, limited liability company, partnership or person or entity submitting a bid for the same materials, supplies, equipment or services and is in all respects fair and without collusion or fraud. I further certify that I am authorized to sign the certification on behalf of the bidder or this bid.

LICENSING:

Vendors must be licensed and in good standing in accordance with any and all state and local laws and requirements by any state or local agency of West Virginia, including, but not limited to, the West Virginia Secretary of State's Office, the West Virginia Tax Department, West Virginia Insurance Commission, or any other state agencies or political subdivision. Furthermore, the vendor must provide all necessary releases to obtain information to enable the Director or spending unit to verify that the vendor is licensed and in good standing with the above entities.

CONFIDENTIALITY:

The vendor agrees that he or she will not disclose to anyone, directly or indirectly, any such personally identifiable information or other confidential information gained from the agency, unless the individual who is the subject of the information consents to the disclosure in writing or the disclosure is made pursuant to the agency's policies, procedures and rules. Vendor further agrees to comply with the Confidentiality Policies and Information Security Accountability Requirements, set forth in http://www.state.wv.us/admin/purchase/privacy/noticeConfidentiality.pdf.

Under penalty of law for false swearing (West Virginia Code §61-5-3), it is hereby certified that the vendor affirms and acknowledges the information in this affidavit and is in compliance with the requirements as stated.

Vendor's Name:		
Authorized Signature:	Date:	

Purchasing Affidavit (Revised 01/01/09)