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State of West Virginia Department of Administration Purchasing Division 2019 Washington Street East Post Office Box 50130 Charleston, WV 25305-0130

### Request for Quotation

DNR70162

ADDRESS:CORRESPONDENCE TO ATTENTION OF:

BUYER 32

304-558-0492

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DIVISION OF NATURAL RESOURCES WILDLIFE RESOURCES SECTION JOBSITE SEE SPECIFICATIONS

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### GENERAL TERMS & CONDITIONS REQUEST FOR QUOTATION (RFQ) AND REQUEST FOR PROPOSAL (RFP)

- 1. Awards will be made in the best interest of the State of West Virginia.
- 2. The State may accept or reject in part, or in whole, any bid.
- 3. All quotations are governed by the West Virginia Code and the Legislative Rules of the Purchasing Division.
- 4. Prior to any award, the apparent successful vendor must be properly registered with the Purchasing Division and have paid the required registration fee. (Effective June 8, 2006, the fee will change from \$45.00 to \$125.00 pursuant to House Bill 4031.)
- 5. All services performed or goods delivered under State Purchase Orders/Contracts are to be continued for the term of the Purchase Order/Contract, contingent upon funds being appropriated by the Legislature or otherwise being made available. In the event funds are not appropriated or otherwise available for these services or goods, this Purchase Order/Contract becomes void and of no effect after June 30.
- 6. Payment may only be made after the delivery and acceptance of goods or services.
- 7. Interest may be paid for late payment in accordance with the West Virginia Code.
- 8. Vendor preference will be granted upon written request in accordance with the West Virginia Code.
- 9. The State of West Virginia is exempt from federal and state taxes and will not pay or reimburse such taxes.
- 10. The Director of Purchasing may cancel any Purchase Order/Contract upon 30 days written notice to the seller.
- 11. The laws of the State of West Virginia and the *Legislative Rules* of the Purchasing Division shall govern all rights and duties under the Contract, including without limitation the validity of this Purchase Order/Contract.
- 12. Any reference to automatic renewal is hereby deleted. The Contract may be renewed only upon mutual written agreement of the parties.
- 13. BANKRUPTCY: In the event the vendor/contractor files for bankruptcy protection, this contract is automatically null and void, and is terminated without further order.
- 14. HIPAA Business Associate Addendum The West Viginia State Government HIPAA Business Associate Addendum (BAA), approved by the Attorney General, and available online at the Purchasing Division's web site (http://www.state.wv.us/admin/purchase/vrc/hipaa.htm) is hereby made part of the agreement. Provided that, the Agency meets the definition of a Covered Entity (45 CFR §160.103) and will be disclosing Protected Health Information (45 CFR §160.103) to the vendor.

### **INSTRUCTIONS TO BIDDERS**

- 1. Use the quotation forms provided by the Purchasing Division.
- 2. SPECIFICATIONS: Items offered must be in compliance with the specifications. Any deviation from the specifications must be clearly indicated by the bidder. Alternates offered by the bidder as EQUAL to the specifications must be clearly defined. A bidder offering an alternate should attach complete specifications and literature to the bid. The Purchasing Division may waive minor deviations to specifications.
- 3. Complete all sections of the quotation form.
- Unit prices shall prevail in cases of discrepancy.
- 5. All quotations are considered F.O.B. destination unless alternate shipping terms are clearly identified in the quotation.
- 6. DUPLICATE BIDS: All quotations must be delivered by the bidder to the respective offices listed below prior to the date and time of the bid opening. Failure of the bidder to deliver the quotations on time will result in bid disqualifications.

### **ORIGINAL SIGNED BID TO:**

Department of Administration Purchasing Division 2019 Washington Street East Post Office Box 50130 Charleston, WV 25305-0130

### **DUPLICATE BID TO:**

State Auditor's Office Bid Observer Building 1 Room W114 1900 Kanawha Boulevard, East Charleston, WV 25305-0230

NOTICE: Beginning June 8, 2006, there is no need to submit a duplicate bid to the State Auditor's Office pursuant to House Bill 4031.

### West Virginia Division of Natural Resources Wildlife Resources Section Spring Run Fish Hatchery Improvements Dorcas, West Virginia

### ADDENDUM NO. 1

### May 22, 2006

### A. REQUEST FOR QUOTATION

- 1. EXHIBIT 5 WAGE RATES Page 3:
  - A. Delete reference to U.S. Department of Labor Minimum Wage Rates for Grant County. This project will require compliance with only State of West Virginia Department of Labor Minimum Wage Rates for Grant County.

### B. SPECIFICATIONS

1. Section 11325 - Sludge Transfer Pumps, page 12 - 13, Spare Parts Kits:

Provide the stated spare parts for only one pump, not each pump as stated, since the pumps are identical.

### C. DRAWINGS

1. Drawing Nos. C3A, C3B, and C8 regarding new electrical service to wastewater treatment system:

Owner will bear the cost of any charges associated with new electrical service and relocation of existing light pole. Contractor to coordinate his work with the electrical company. New electrical service drop pole will be supplied by the electric company.

### D. GENERAL

- 1. The Owner has stated that the value of fish if lost or damaged due to the Contractor's work will be approximately \$2.50 per pound.
- 2. Geotechnical Engineering Report prepared by Novel Geo-Environmental, PLLC, dated May 2006, is included herewith for information only, and is not to be considered as part of the contract documents. Bidder/Contractor may not rely upon or make any claim against Owner, Engineer, or any of Engineer's Consultants with respect to any Bidder/Contractor interpretation of or conclusion drawn from this information.

- 3. Pre-Bid Attendance roster is included herewith.
- 4. Bidders are hereby notified to acknowledge receipt of this addendum in the space provided on the Form of Proposal.

No. 13093

CHAPMAN TECHNICAL GROUP

Robert G. Belcher, P.E.

Vice President, Engineering

RGB/ktc h:/projects/04053/addendum/addend no-1.doc

## West Virginia Division of Natural Resources - Wildlife Resources Section SPRING RUN FISH HATCHERY IMPROVEMENTS

# MANDATORY PRE-BID CONFERENCE ATTENDANCE LIST May 16, 2006 @ 1:00 P.M.

Phone No. Fax No.	304) 722- 4237   201 - 2405	001	6501-156 (405) 1878-156 (405)	717 442-8285 117 442-9330					
•	05, 1NC, (30)		Bryco Bore + Pipe Inc, (304)25		60 27.41 P.D. BOX 549	GAP, PA 17527			
Name	GRIFFIN Mª CABE	7	Valerie Laub	10+					



GEOTECHNICAL INVESTIGATION PROPOSED MODIFICATIONS SPRING RUN FISH HATCHERY GRANT COUNTY, WEST VIRGINIA

Novel Geo-Environmental, PLLC

Pittsburgh, Pennsylvania Charleston, West Virgina

### GEOTECHNICAL INVESTIGATION PROPOSED MODIFICATIONS SPRING RUN FISH HATCHERY GRANT COUNTY, WEST VIRGINIA

**NOVEL GEO-ENVIRONMENTAL PROJECT NO. W06035** 

### SUBMITTED TO:

CHAPMAN TECHNICAL GROUP GRANT COUNTY, WEST VIRGINIA

### **SUBMITTED BY:**

NOVEL GEO-ENVIRONMENTAL, PLLC ST. ALBANS, WEST VIRGINIA

**MAY 2006** 



### Novel Geo-Environmental, PLLC

806 B Street • St. Albans, WV 25177 304-201-5180 • Fax 304-201-5182

May 16, 2006

Mr. Jeffery D. Ekstrom, P.E. Chapman Technical Group 200 Sixth Avenue St. Albans, WV 25177

Subject:

**GEOTECHNICAL INVESTIGATION** 

Proposed Spring Run Fish Hatchery Modifications

Grant County, West Virginia

Novel Geo-Environmental Project No. W06035

Dear Mr. Ekstrom:

In accordance with your request, we have performed a geotechnical investigation for the subject site in Grant County, West Virginia. Authorization to proceed with this project was provided by execution of Novel Proposal No. PW06522.

This report presents the results of the field and laboratory investigation performed to determine the subsurface conditions, as well as our conclusions and recommendations concerning the geotechnical considerations for the site.

We appreciate the opportunity to assist you on this project and trust this report satisfies your needs at this time. Please feel free to contact us if you have any questions concerning this report, or if we can provide any further assistance.

Sincerely,

NOVEL GEO-ENVIRONMENTAL, PLLC

Charles E. Montgomery, P.G.

**Project Geologist** 

John E. Nottingham, P.E

Principal Engineer

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**FIGURES** 

### 1.0 PROJECT DESCRIPTION

The subsurface investigation was performed for proposed modifications to the existing Spring Run Fish Hatchery in Grant County, West Virginia. The purpose of the investigation was to determine subsurface conditions and provide geotechnical recommendations. According to information provided by the client, the modifications will consist of the following:

- Three new raceways in the area of the existing rearing ponds. The raceways will consist of cast-in-place concrete structures approximately 3.3 ft. deep and 170 ft. long.
- A new sludge holding tank, approximately 20 ft. in diameter and 14 ft.
   deep. The tank will be a below-grade concrete structure.
- A new clarifier, also a below-grade concrete structure. The clarifier will be approximately 25 ft. in diameter and extend 19.6 ft. below the ground surface.

### 2.0 DRILLING AND SAMPLING PROCEDURES

A total of four test borings (Borings B-1 through B-4) were drilled to determine subsurface conditions at the proposed fish hatchery modifications. The test borings were drilled using a truck mounted rotary drilling rig equipped with 3-1/4 inch I.D. hollow stem augers. Standard penetration testing and sampling was performed at 2.5 ft. intervals from the ground surface to the boring termination depth or refusal on bedrock in each of the borings. The standard penetration testing and sampling was performed in accordance with ASTM D-1586. In addition, rock coring was performed in Borings B-1 and B-2 following auger refusal on bedrock.

Standard penetration testing is performed by driving a 2.0 inch O.D. split-barrel sampler into the soil with a 140-lb. hammer dropping a distance of 30 inches. The sampler is driven a distance of 18 inches in three 6-inch increments, and the number of blows required to produce the last two 6-inch increments of penetration is termed the

Standard Penetration Number or "N" value. These values provide an indication of the consistency or relative density of the soils.

A 1-3/8 inch diameter soil sample was obtained from the boring in conjunction with each penetration test. All standard penetration samples were placed in air-tight glass jars. Two inch diameter core samples of the bedrock were placed in partitioned wooden boxes. Upon completion of drilling, all samples were delivered to our laboratory where they were examined by a geotechnical engineer. Soil and bedrock descriptions, standard penetration numbers, and other pertinent subsurface information are provided on the boring logs (Figure Nos. 2 through 5) in the back of this report.

### 3.0 SUBSURFACE CONDITIONS

Test boring logs providing detailed information at each exploration point are provided in the back of this report (Figure Nos. 2 through 5). A summary of the subsurface conditions encountered in the test borings is as follows:

### 3.1 SOIL AND BEDROCK CONDITIONS

Soil overburden at the site consisted primarily of natural silty to sandy clay with varying degrees of rock fragments. The clayey soil was encountered immediately below topsoil in Borings B-1 and B-2 and beneath a surface layer of crushed stone aggregate in Borings B-3 and B-4. Standard penetration N-values within the natural clay were between 6 and 21 blows per foot of penetration, indicating a medium stiff to very stiff cohesive soil condition. A higher penetration value of 50 blows per foot was obtained in Boring B-4 at a depth of five feet due to the presence of large cobbles and/or boulders within the soil.

Natural moisture content values with the clay soil varied between approximately 13 and 28 percent. Atterberg limits testing of two samples indicates the material is of medium to high plasticity. A representative sample from Boring B-2 exhibited a plasticity index (PI) of 25 percent. A sample from Boring B-4 yielded a PI of 13 percent.

The natural clay extended to the top of bedrock in Borings B-1, B-2, and B-4, and to the boring termination depth in Boring B-3.

Bedrock was encountered below the clay in Borings B-1, B-2, and B-4 at depths ranging from 7.5 to 10.3 ft. below the existing ground surface. Bedrock consisted of medium hard siltstone that exhibited a moderate to high degree of fracturing and contained occasional calcite laminations.

### 3.2 GROUNDWATER CONDITIONS

Groundwater was noted at a depth of approximately 10.0 ft. during drilling operations in Borings B-1. The remainder of the borings were noted to be dry during standard penetration sampling. Borings B-1, B-3, and B-4 were left open overnight following completion. The following morning, water was noted in Boring B-1 at a depth of 8.0 ft. In Borings B-3 and B-4, water was measured at 8.5 and 1.8 ft., respectively. It should be noted that the boreholes had partially collapsed in these three borings after the augers were removed. Additionally, groundwater levels typically fluctuate and are generally dependent upon climatic conditions. Groundwater conditions at the time of construction may differ from those observed during our investigation. The test borings were backfilled with auger cuttings prior to departing the site.

### 4.0 CONCLUSIONS AND RECOMMENDATIONS

### 4.1 SITE PREPARATION RECOMMENDATIONS

All topsoil and vegetation should be removed before beginning construction activities. All existing structures and underground utilities should be removed or relocated. Any voids created by removal of structures, utilities, or other underground items should be properly backfilled in accordance with Section 4.2 of this report.

### 4.2 FILL AND BACKFILL RECOMMENDATIONS

All fill or backfill should be placed in maximum 9-inch loose lifts and compacted to 95% of the maximum dry density as determined by the standard Proctor laboratory test (ASTM D-698). Each layer of fill or backfill should be tested by a qualified geotechnical engineering firm to determine that adequate compaction has been achieved prior to placement of additional fill lifts. Fill or backfill should consist of nonorganic soil/rock material with a maximum particle size of 4 inches in any direction. Cohesive soil material to be used as structural fill should have a plasticity index not greater than 16 percent. Due to its high plasticity, we recommend the clay soil in the area of Boring B-2 not be used as fill or backfill around the below grade structures. The moisture content of all fill material should be within three percent of the optimum moisture content as determined by a standard Proctor test.

### 4.3 EXCAVATION CONSIDERATIONS

Any excavation in which workers are required to enter must be properly shored or sloped in accordance with OSHA regulations to provide for worker safety. Based on proposed bearing depths, excavations extending into bedrock will be required for the sludge holding tank and clarifier structures. These excavations will likely require the use of hard rock excavation techniques such as jack-hammering or blasting. Contractors should be aware of the presence of bedrock and be instructed to perform their own investigation/assessment as to the methods necessary for excavation.

### 4.4 RACEWAY CONSTRUCTION

The base of the new raceways will be constructed approximately 3.3 ft. below existing grade. It is assumed that the existing concrete rearing ponds will be demolished and removed prior to beginning raceway construction. We anticipate the raceway bottom will bear on natural clay soil and possibly backfill soil placed after removal of the rearing ponds. We recommend backfill soil placed in this area be of similar composition as the natural soil to facilitate uniform bearing conditions for the raceway structures. Prior to placing concrete, the base of the raceway excavations

should be compacted with a moderately sized roller. If any soft zones are discovered during rolling of the excavation bottom, these areas should be undercut to a firm level and backfilled in accordance with Section 4.2 of this report. Following compaction of the soil subgrade, we recommend a minimum of 4 inches of crushed base stone be placed to provide uniform support and a leveling surface for concrete slab construction. We recommend using a modulus of subgrade reaction ( $K_s$ ) of 150 pci for concrete slab design. Concrete should be placed as soon as possible after completion of the excavation work to reduce softening of the bearing surface due to precipitation. Any water which enters the excavation should be promptly removed by pumping from a sump pit in the base of the excavation.

### 4.5 CLARIFIER AND SLUDGE HOLDING TANK

The excavations for these structures will extend through the existing natural clay soil and into the underlying siltstone bedrock. The siltstone bedrock will provide adequate support for the new clarifier and sludge holding tank. We recommend a maximum allowable bearing pressure of 10,000 psf be used for design. Total and differential settlement of the clarifier and sludge holding tank bearing on bedrock should be negligible.

### 4.6 LATERAL EARTH PRESSURE RECOMMENDATIONS

The walls of below grade structures will be subjected to lateral earth pressure from the backfill soil and natural ground. We recommend that below grade walls be designed and constructed as non-yielding retaining structures using the "at-rest" earth pressure coefficient (K<sub>o</sub>). As previously discussed, high plasticity clay soils (cohesive soil with a plasticity index greater than 16 percent) should not be used as backfill around below grade walls. Assuming that low plasticity clayey soil will be used as wall backfill materials, we recommend the parameters provided in the following table be used for wall design:

### **Below Grade Wall Design Parameters**

PARAMETER	RECOMMENDED VALUE							
At Rest Earth Pressure Coefficient	0.60							
Soil Unit Weight	130 pcf							

Below grade walls should also be designed considering potential additional surcharge loads exerted nearby such as adjacent structures, traffic loading, etc. Any surcharge loads anticipated at the surface should be multiplied by 0.5 and superimposed as a uniform horizontal pressure in addition to the recommended at-rest lateral earth pressure. In addition to surcharge loads, below grade walls should be designed to resist hydrostatic pressures resulting from high groundwater that could develop during flood conditions.

### 4.7 BUOYANT FORCES

Below grade structures may be subjected to uplifting buoyant forces from groundwater. The designer of these structures should consider potential groundwater conditions approximately equal to the water level from the design flood. If the dead and live weight of any structure is not sufficient to resist potential buoyant forces, we recommend a system of concrete deadmen or tie-down anchors be used.

### 4.8 ENGINEERING INSPECTIONS AND QUALITY ASSURANCE

Fill placement and compaction should be monitored by a qualified geotechnical engineering firm to verify the suitability of the fill and that compaction requirements are met. Foundation construction should be inspected by our geotechnical engineer to verify the adequacy of the bearing materials.

### 5.0 LIMITATIONS

- 1. This work has been prepared for the exclusive use of Chapman Technical Group for use in planning and design of the proposed modifications to the Spring Run Fish Hatchery located in Grant County, West Virginia. The work has been performed in accordance with generally accepted geotechnical engineering practices. No other warranty, expressed or implied, is made.
- 2. In the event that changes in the nature, design or location of the proposed structures is planned, the conclusions and recommendations presented in this report should not be considered valid unless we have reviewed the changes and modified or verified our conclusions and recommendations.
- 3. The conclusions and recommendations contained in this report are based in part on the data obtained from the test borings and our field observations. The nature and extent of the variations between borings and observation locations may not be evident until construction. If variations become evident during construction, we should be contacted in order that actual conditions can be reviewed and applicable conclusions and recommendations can be re-evaluated.

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	- 		Location: See Figure	<u> </u>							<u> </u>		
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feet	Sample Type	OSC		Challer T. I		ıry %	۵	ation	Clay %	FL(70)			(10)
Depth, feet	ple	/ 100	Split Spoon	Shelby Tul		Recovery	RGD	netra s / 6	) pu				
Det	Sam	Symbol / USCS	Rock Core	Bag Samp	le ———	Rec		Penetration Blows / 6 inches	Silt and (		<b>⊚</b> SPT bp	of	
			MATERIAL	DESCRIPTION						10	20 3	0 40	)
L	$\mathbb{X}$		GRAVEL					8-8-8			<b>&gt;</b> \$		
	$\vdash$		Brown and gray SILTY to	SANDY CLAY with							/		
<b>†</b> -	<u> </u>		rock fragments, moist, m	edium stiff to hard						ΓΧ			٦
<b>-</b>	X							2-3-3			<del> </del>	4	-
<b>-</b>	$\vdash$									F [			-
- 5 -	<u></u>												
L	$\mathbb{X}$							5-29-21			þ		
Γ	$\sim$		<ul> <li>w/cobbles and boulders</li> </ul>	s from 6.0 ft.									
<b> </b>	_												7
-	$\mathbb{N}$							8-7-6		-			-
<b>-</b>	$\swarrow$									F			-
-10-	<u> </u>				10.3								
"	$\mathbb{N}$		Gray SILTSTONE, media	ım hard, weathered	10.8	1		18-50/4"					>>[
	$\sim$		Bottom of Test I	3oring @ 10.8 ft.									
<b>†</b> -	1												-
<u> </u>	1									<b> </b>			-
<b>L</b> -	-									-			-
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5/16/06										<b> </b>			_
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Š _	4									-			_
5.GP.										-			
7,0003													
\$ 8	mplet	ion De	pth: 10.8 ft.	Remarks: Borin	ıg was n	oted to	be d	ry during dri	lling	operations	and boring c	ompletio	n.
₹ Da	te Bo	ring St	arted: 5/3/06 ompleted: 5/3/06										
En		r/Geol	ogist: <b>JEN/CEM</b>										
ŏ Dri	ller:		NOVEL	Depth to Water (	24 hrs.	:							