

Request for Quotation

DEFK9020

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JOHN ABBOTT
304-558-2544

RFQ COPY TYPE NAME/ADDRESS HERE

Thaxton Construction Co., Inc. P.O. Box 13279 Charleston, WV 25360

DIV ENGINEERING & FACILITIES CAMP DAWSON ARMY TRAINING SITE 240 ARMY ROAD

KINGWOOD, WV 26537-1077

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GENERAL TERMS & CONDITIONS REQUEST FOR QUOTATION (RFQ) AND REQUEST FOR PROPOSAL (RFP)

- 1. Awards will be made in the best interest of the State of West Virginia.
- 2. The State may accept or reject in part, or in whole, any bid.
- 3. All quotations are governed by the West Virginia Code and the Legislative Rules of the Purchasing Division.
- 4. Prior to any award, the apparent successful vendor must be properly registered with the Purchasing Division and have paid the required \$125 fee.
- 5. All services performed or goods delivered under State Purchase Order/Contracts are to be continued for the term of the Purchase Order/Contracts, contingent upon funds being appropriated by the Legislature or otherwise being made available. In the event funds are not appropriated or otherwise available for these services or goods, this Purchase Order/Contract becomes void and of no effect after June 30.
- 6. Payment may only be made after the delivery and acceptance of goods or services.
- 7. Interest may be paid for late payment in accordance with the West Virginia Code.
- 8. Vendor preference will be granted upon written request in accordance with the West Virginia Code.
- 9. The State of West Virginia is exempt from federal and state taxes and will not pay or reimburse such taxes.
- 10. The Director of Purchasing may cancel any Purchase Order/Contract upon 30 days written notice to the seller.
- 11. The laws of the State of West Virginia and the *Legislative Rules* of the Purchasing Division shall govern all rights and duties under the Contract, including without limitation the validity of this Purchase Order/Contract.
- 12. Any reference to automatic renewal is hereby deleted. The Contract may be renewed only upon mutual written agreement of the parties.
- 13. BANKRUPTCY: In the event the vendor/contractor files for bankruptcy protection, this Contract may be deemed null and void, and terminated without further order.
- 14. HIPAA BUSINESS ASSOCIATE ADDENDUM: The West Virginia State Government HIPAA Business Associate Addendum (BAA), approved by the Attorney General, and available online at the Purchasing Division's web site (http://www.state.wv.us/admin/purchase/vrc/hipaa.htm) is hereby made part of the agreement. Provided that, the Agency meets the definition of a Cover Entity (45 CFR §160.103) and will be disclosing Protected Health Information (45 CFR §160.103) to the vendor.
- 15. WEST VIRGINIA ALCOHOL & DRUG-FREE WORKPLACE ACT: If this Contract constitutes a public improvement construction contract as set forth in Article 1D, Chapter 21 of the West Virginia Code ("The West Virginia Alcohol and Drug-Free Workplace Act"), then the following language shall hereby become part of this Contract: "The contractor and its subcontractors shall implement and maintain a written drug-free workplace policy in compliance with the West Virginia Alcohol and Drug-Free Workplace Act, as set forth in Article 1D, Chapter 21 of the West Virginia Code. The contractor and its subcontractors shall provide a sworn statement in writing, under the penalties of perjury, that they maintain a valid drug-free work place policy in compliance with the West Virginia and Drug-Free Workplace Act. It is understood and agreed that this Contract shall be cancelled by the awarding authority if the Contractor: 1) Fails to implement its drug-free workplace policy; 2) Fails to provide information regarding implementation of the contractor's drug-free workplace policy at the request of the public authority; or 3) Provides to the public authority false information regarding the contractor's drug-free workplace policy."

INSTRUCTIONS TO BIDDERS

- 1. Use the quotation forms provided by the Purchasing Division.
- 2. SPECIFICATIONS: Items offered must be in compliance with the specifications. Any deviation from the specifications must be clearly indicated by the bidder. Alternates offered by the bidder as EQUAL to the specifications must be clearly defined. A bidder offering an alternate should attach complete specifications and literature to the bid. The Purchasing Division may waive minor deviations to specifications.
- 3. Complete all sections of the quotation form.
- 4. Unit prices shall prevail in case of discrepancy.
- 5. All quotations are considered F.O.B. destination unless alternate shipping terms are clearly identified in the quotation.
- 6. BID SUBMISSION: All quotations must be delivered by the bidder to the office listed below prior to the date and time of the bid opening. Failure of the bidder to deliver the quotations on time will result in bid disqualifications: Department of Administration, Purchasing Division, 2019 Washington Street East, P.O. Box 50130, Charleston, WV 25305-0130



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State of West Virginia Department of Administration
Purchasing Division
2019 Washington Street East
Post Office Box 50130 Charleston, WV 25305-0130

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JOHN	ABBOTT	-	
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DIV ENGINEERING & FACILITIES CAMP DAWSON ARMY TRAINING SITE 240 ARMY ROAD

KINGWOOD, WV 26537-1077

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State of West Virginia
Department of Administration
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2019 Washington Street East
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Request for Quotation DEFK9020

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JOHN ABBOTT 304-558-2544

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Request for Quotation

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ADDRESS:CORRESPONDENCE TO ATTENTION OF

JOHN ABBOTT 304-558-2544

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DIV ENGINEERING & FACILITIES CAMP DAWSON ARMY TRAINING SITE 240 ARMY ROAD

KINGWOOD, WV 26537-1077

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JOHN ABBOTT 304-558-2544

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KINGWOOD, WV 26537-1077

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Purchasing Division
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State of West Virginia Department of Administration Purchasing Division 2019 Washington Street East Post Office Box 50130 Charleston, WV 25305-0130

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JOHN	ABBOTT	

304-558-2544

DIV ENGINEERING & FACILITIES CAMP DAWSON ARMY TRAINING SITE 240 ARMY ROAD

KINGWOOD, WV 26537-1077

304-329-4417

F.O.B. FREIGHT/TERMS SHIP VIA DATE:PRINTED TERMS OF SALE 03/12/2009 BID OPENING TIME 01:30PM BID OPENING DATE: 04/15/2009 AMOUNT UNITPRICE ITEM NUMBER UOP QUANTITY LINE CALLY NULL AND VOID, AND IS TERMINATED WITHOUT FURTHER ORDER. REV. 1/2005 NOTICE A SIGNED BID MUST BE SUBMITTED TO: DEPARTMENT OF ADMINISTRATION PURCHASING DIVISION BUILDING 15 2019 WASHINGTON STREET, EAST CHARLESTON, WV 25305-0130 THE BID SHOULD CONTAIN THIS INFORMATION ON THE FACE OF THE ENVELOPE OR THE BID MAY NOT BE CONSIDERED: SEALED BID JOHN ABBOTT---BUYER: DEFK9020----REQ. NO.: BID OPENING DATE: 04/15/2009----BID OPENING TIME: 1:30 PM----PLEASE PROVIDE A FAX NUMBER IN CASE IT IS NECESSARY TO CONTACT YOU REGARDING YOUR BID: 984-2334 PLEASE PRINT OR TYPE NAME OF PERSON TO CONTACT SEE REVERSE SIDE FOR TERMS AND CONDITIONS SIGNATURE 984-2299 4/22/09 ADDRESS CHANGES TO BE NOTED ABOVE 4/22/09 **President**



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Project Name: WVARNG South Gate Project No. <u>7-7728-0000</u> Chk'd By: DWD

WVARNG South Gate Road Slope Repair Design Brief

Dated: 2/25/09

Prepared For:

WVARNG



Prepared By:





Project Name:	WVARNG South Gate
Road	4 .
Project No7-	7728-0000
D. CID	
By: <u>CJR</u>	מעוכ
Chk'd By:I	JVV D

Date: 12/04/08

Design Discussion

The South Gate Access Road has suffered from intermittent erosion and minor landslides, which have led to a state of disrepair and limited use. AMEC prepared three concepts for stabilization of approximately 230 linear feet of the road. AMEC recommended, and WVARNG concurred that the best method of repair was a drilled shaft wall embedded into bedrock with precast concrete lagging spanning between the shafts near the surface. Multiple exploratory borings and surface reconnaissance were used to estimate the limits of the proposed drilled shaft retaining wall, the loads that would be induced on the system as it supports the restored road grade and adjacent hillside, and the soil & rock properties within the underlying subsurface profile.

Lab tests indicate an average unit weight for the overburden soils is approximately 115 pounds per cubic foot (pcf). Considering the amount of clay content in soil matrix, an appropriate friction angle would be 25 degrees. Field and lab inspection of the rock core samples indicate a hard shale, that would most likely not be easily removed with normal augering tools. As such, we assigned a unit weight of 140 pcf, a friction angle of 40 degrees and a shear strength of 10,000 pounds per square foot to the rock

Drilled shaft retaining walls are common in roadsides next to river valley applications, similar to this. The methodology developed to design this system is derived from the following sources:

- AASHTO Standard Specifications for Bridges, 17th Edition
- FHWA Publication Geotechnical Engineering Circular No. 4 (1999), and
- "Slide Control by Drilled Pier Walls", M. Nethero, ASCE National Convention; Las Vegas, NV; 1982.



Project Name: <u>WVARNG South Gate</u> Road Project No. _7-7728-0000

CJR Chk'd By: DWD

Drilled Shaft Retaining Wall - Design Section 11+00:

Soil Parameters:

Unit weight

 $\gamma_{\text{soil}} := 115 \cdot \text{pcf}$

Phi angle $\phi_{\text{soil}} := 25 \cdot \text{deg}$ $k_{\text{a_soil}} := \tan \left(45 \cdot \text{deg} - \frac{\phi_{\text{soil}}}{2} \right)^2$ $k_{\text{a_soil}} = 0.41$ FS := 1.5 $k_{\text{p_soil}} := \frac{1}{k_{\text{a_soil}}}$ $k_{\text{p_soil}} = 2.46$

Factor of Safety on Passive

FS := 1.5

Rock Parameters:

Unit weight

 $\gamma_{\text{rock}} := 140 \cdot \text{pcf}$ Phi angle

 $\phi_{\text{rock}} := 40 \cdot \text{deg}$

 $k_{a_rock} := tan \left(45 \cdot deg - \frac{\phi_{rock}}{2} \right)^2$

 $k_{a_rock} = 0.22$

Factor of Safety on Passive

FS := 1.5

 $k_{p_rock} := \frac{1}{k_{a_rock}} \qquad k_{p_rock} = 4.60$

Rock Shear Strength

 $C_{rock} := 10 ksf$

Wall Characteristics:

Wall height

 $H := 27 \cdot ft$

Depth of Overburden h := 18ft

Pile Width

 $b_f = 2.5 \cdot ft$ Pile spacing

 $s := 10 \cdot ft$

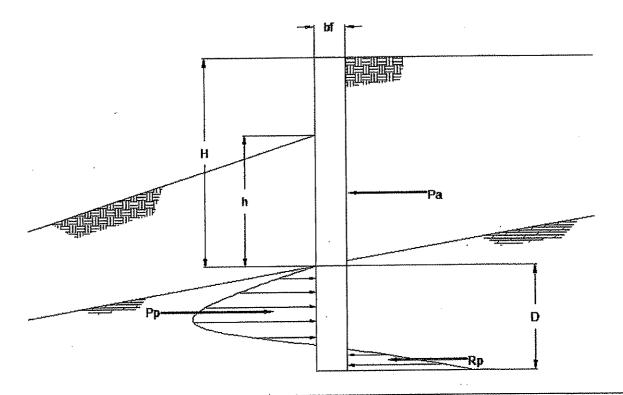
Horizontal Surcharge Pressure applied

q := 100psf

 $b_e := if(3 \cdot b_f < s, 3 \cdot b_f, s)$ $b_e = 7.5 ft$

Depth of Rock Socket

D := 12ft





By: <u>CJR</u> Chk'd By: <u>DWD</u>

$$P_a := k_{a_soil} \cdot \gamma_{soil} \cdot \frac{(H)^2}{2} = 17.01 \cdot \frac{kips}{ft}$$

SUM MOMENTS ABOUT "P" AND SOLVE FOR "Rp"

$$R_p := \left[\left(P_{a} \cdot s \right) \cdot \left(\frac{H}{3} + \frac{D}{3} \right) - \gamma_{soil} \cdot \left[\frac{k_{p_soil} \cdot b_e}{FS} \cdot \left[\frac{\left(h\right)^2}{2} \right] \right] \cdot \left(\frac{h}{3} + \frac{D}{3} \right) + q \cdot H \cdot s \cdot \left(\left(\frac{H}{2} + \frac{D}{3} \right) \right) \right] \cdot \frac{9}{D \cdot 5} = 58.35 \cdot \text{kips}$$

SUM OF THE HORIZONTAL FORCES = 0, DETERMINE "Pp"

$$P_{p} := P_{a} \cdot s - \left[\gamma_{soil} \cdot \frac{k_{p_soil} \cdot b_{e}}{FS} \cdot \left(\frac{h^{2}}{2} \right) \right] + q \cdot H \cdot s + R_{p} = 25.96 \text{ kips}$$

$$P_{p_avg} := \frac{P_p}{\frac{2 \cdot D \cdot b_f}{3}} = 1.3 \cdot ksf$$

$$P_{p_{max}} := 1.5 \cdot P_{p_{avg}} = 1.95 \cdot ksf$$

$$R_{p_avg} := \frac{R_p}{D \cdot \frac{b_f}{3}} = 5.83 \cdot ksf$$

$$R_{p_max} := 2 \cdot R_{p_avg} = 11.67 \cdot ksf$$

DUE TO TRIANGULAR DISTRIBUTION



Project Name: <u>WVARNG South Gate</u> Project No. <u>7-7728-0</u>000

CJR Chk'd By: DWD

ULTIMATE PASSIVE PRESSURE @ Pp or D/3

$$D3_{pass} := \gamma_{soil} \cdot H + 2 \cdot C_{rock} + \gamma_{rock} \cdot \frac{D}{3} = 23.66 \cdot ksf$$

$$FS_{D3_pass} := \frac{D3_{pass}}{P_{p_max}} = 12.15$$

2.0

OK!

ULTIMATE PASSIVE PRESSURE @ Rp or 8D/9

$$D_{8_9pass} := \gamma_{soil} \cdot H + 2 \cdot C_{rock} + \gamma_{rock} \cdot \frac{8D}{9} = 24.6 \cdot ksf$$

$$FS_{D8_9pass} := \frac{D_{8_9pass}}{R_{p_avg}} = 4.22$$

2.0

OK!

ULTIMATE PASSIVE PRESSURE @ D

$$D_{pass} := \gamma_{soil} \cdot H + 2 \cdot C_{rock} + \gamma_{rock} \cdot D = 24.79 \cdot ksf$$

$$FS_{Dpass} := \frac{D_{pass}}{R_{p_max}} = 2.12$$

2.0

OK!

FIND POINT OF ZERO SHEAR

$$V_{o} \coloneqq \frac{\left[P_{a} \cdot s - \left[\gamma_{soil} \cdot \frac{k_{p_soil} \cdot b_{e}}{FS} \cdot \left(\frac{h^{2}}{2}\right)\right] + q \cdot H \cdot s\right]}{b_{f} \cdot P_{p_avg}} = -9.98 \, ft \qquad \text{ABOVE TOP OF ROCK}$$

MAXIMUM MOMENT

$$M_{\text{max}} := \left(\frac{H}{3} + V_{\text{o}}\right) \cdot P_{\text{a}} \cdot s - \left(\frac{h}{3} + V_{\text{o}}\right) \left[\gamma_{\text{soil}} \cdot \frac{k_{\text{p_soil}} \cdot b_{\text{e}}}{FS} \cdot \left(\frac{h^2}{2}\right)\right] + \left(\frac{H}{2} + V_{\text{o}}\right) \cdot q \cdot H \cdot s - P_{\text{p_avg}} \cdot b_{\text{f}} \cdot \frac{V_{\text{o}}^2}{2} = 680.16 \cdot \text{ft-kips}$$

$$F_a := .66F_y = 33 \cdot ksi$$

$$S_{x_rqd} := \frac{M_{max}}{F_a} = 247.33 \cdot in^3$$

USE W21x111 GR50 PILE



Project Name: WVARNG South Gate Road

Project No. <u>7-7728-0000</u>

DWD Chk'd By:

Drilled Shaft Retaining Wall - Design Sections 12+00 & 13+00:

Soil Parameters:

Unit weight

 $\gamma_{\text{soil}} := 115 \cdot \text{pcf}$

Phi angle

 $\phi_{\text{soil}} := 25 \cdot \text{deg}$

 $k_{a_soil} := tan \left(45 \cdot deg - \frac{\phi_{soil}}{2}\right)^2$ $k_{a_soil} = 0.41$

Factor of Safety on Passive

FS := 1.5

Rock Parameters:

Unit weight

 $\gamma_{\text{rock}} := 140 \cdot \text{pcf}$.

Phi angle

 $\Phi_{\text{rock}} := 40 \cdot \text{deg}$ $k_{\text{a_rock}} := \tan \left(45 \cdot \text{deg} - \frac{\Phi_{\text{rock}}}{2} \right)^{2}$

Factor of Safety on Passive

FS := 1.5

 $k_{p_rock} := \frac{1}{k_{p_rock}} \qquad k_{p_rock} = 4.60$

Rock Shear Strength

 $C_{rock} := 10ksf$

Wall Characteristics:

Wall height

H := 20 ft

Depth of Overburden h := 11ft

Pile Width

 $b_f := 2.5 \cdot ft$ Pile spacing

 $s := 10 \cdot ft$

Horizontal Surcharge Pressure applied

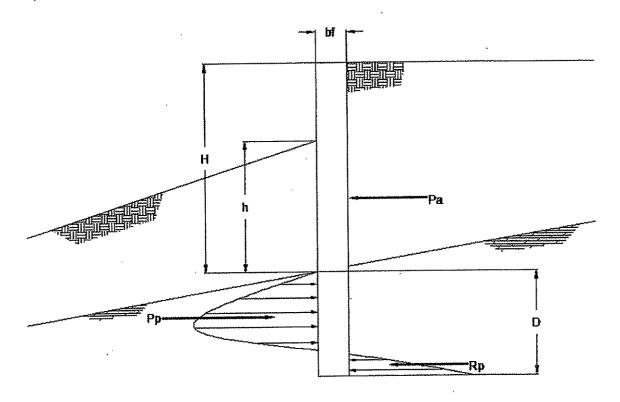
q := 100psf

 $b_e := if(3 \cdot b_f < s, 3 \cdot b_f, s)$

 $b_e = 7.5 \, ft$

Depth of Rock Socket

D := 15ft





Project Name: <u>WVARNG South Gate</u>
Road
Project No. <u>7-7728-0000</u>

By: <u>CJR</u> Chk'd By: <u>DWD</u>

$$P_a := k_{a_soil} \cdot \gamma_{soil} \cdot \frac{(H)^2}{2} = 9.33 \cdot \frac{kips}{ft}$$

SUM MOMENTS ABOUT "P" AND SOLVE FOR "Rp"

$$R_p := \left[\left(P_{a} \cdot s \right) \cdot \left(\frac{H}{3} + \frac{D}{3} \right) - \gamma_{soil} \cdot b_e \cdot \left[\frac{k_{p_soil}}{FS} \left[\frac{\left(h \right)^2}{2} \right] \right] \cdot \left(\frac{h}{3} + \frac{D}{3} \right) + q \cdot H \cdot s \cdot \left(\left(\frac{H}{2} + \frac{D}{3} \right) \right) \right] \cdot \frac{9}{D \cdot 5} = 77.54 \cdot kips$$

SUM OF THE HORIZONTAL FORCES = 0, DETERMINE "Pp"

$$P_{p} := P_{a} \cdot s - \left[\gamma_{soil} \cdot b_{e} \cdot \frac{k_{p_soil}}{FS} \cdot \left(\frac{h^{2}}{2} \right) \right] + q \cdot H \cdot s + R_{p} = 105.18 \cdot kips$$

$$P_{p_avg} := \frac{P_p}{\frac{2 \cdot D \cdot b_f}{3}} = 4.21 \cdot ksf$$

$$P_{p_{max}} := 1.5 \cdot P_{p_{avg}} = 6.31 \cdot \text{ksf}$$

$$R_{p_avg} := \frac{R_p}{D \cdot \frac{b_f}{2}} = 6.2 \cdot ksf$$

$$R_{p_{\text{max}}} := 2 \cdot R_{p_{\text{avg}}} = 12.41 \cdot \text{ksf}$$

DUE TO TRIANGULAR DISTRIBUTION



Project Name: <u>WVARNG South Gate</u>
Road

Project No. <u>7-7728-0000</u>

By: <u>CJR</u> Chk'd By: <u>DWD</u>

ULTIMATE PASSIVE PRESSURE @ Pp or D/3

$$D3_{pass} := \gamma_{soif} \cdot H + 2 \cdot C_{rock} + \gamma_{rock} \cdot \frac{D}{3} = 23 \cdot ksf$$

$$FS_{D3_pass} := \frac{D3_{pass}}{P_{p_{max}}} = 3.64$$

2.0

OK!

ULTIMATE PASSIVE PRESSURE @ Rp or 8D/9

$$D_{8_9pass} := \gamma_{soil} \cdot H + 2 \cdot C_{rock} + \gamma_{rock} \cdot \frac{8D}{9} = 24.17 \cdot ksf$$

$$FS_{D8_9pass} := \frac{D_{8_9pass}}{R_{p avg}} = 3.9$$

2.0

OK!

ULTIMATE PASSIVE PRESSURE @ D

$$D_{\text{pass}} := \gamma_{\text{soil}} \cdot H + 2 \cdot C_{\text{rock}} + \gamma_{\text{rock}} \cdot D = 24.4 \cdot \text{ksf}$$

$$FS_{Dpass} := \frac{D_{pass}}{R_{p_max}} = 1.97$$

2.0

OK!

FIND POINT OF ZERO SHEAR

$$V_o := \frac{\left[P_a \cdot s - \left[\gamma_{soil} \cdot b_e \cdot \frac{k_{p_soil}}{FS} \cdot \left(\frac{h^2}{2}\right)\right] + q \cdot H \cdot s\right]}{b_f \cdot P_{p_avg}} = 2.63 \, ft$$

ABOVE TOP OF ROCK

MAXIMUM MOMENT

$$M_{max} := \left(\frac{H}{3} + V_o\right) \cdot P_a \cdot s - \left(\frac{h}{3} + V_o\right) \cdot \left[\gamma_{soil} \cdot b_e \cdot \frac{k_{p_soil}}{FS} \cdot \left(\frac{h^2}{2}\right)\right] + \left(\frac{H}{2} + V_o\right) \cdot q \cdot H \cdot s - P_{p_avg} \cdot b_f \cdot \frac{{V_o}^2}{2} = 544.34 \cdot ft \cdot kips$$

$$F_v := 50$$
ksi

$$F_a := .66F_y = 33 \cdot ksi$$

$$S_{x_rqd} := \frac{M_{max}}{F_a} = 197.94 \cdot in^3$$

USE W18x106 GR50 PILE



Project Name: <u>WVARNG South Gate</u>

Project No. <u>7-7728-0000</u>

CJR Chk'd By: DWD

Precast Lagging Design:

$$w := \frac{P_{a_max}}{u} + q = 950.5 \cdot psf$$

Clear Span of Lagging ---> $S_{clr} := s - 2t_{brg} = 9.17 \, ft$

$$t_{brg} := 5in$$

Spacing = 9·in

$$M_{\text{max_lag}} := w \cdot \frac{S_{\text{chr}}^2 \cdot 1 \text{ ft}}{10} = 7.99 \cdot \text{ft \cdot kips}$$

$$V_{\text{max_lag}} := w \cdot \frac{S_{\text{clr}} \cdot 1 \text{ft}}{2} = 4.36 \cdot \text{kips}$$

TRY 8" THICK LAGGING

$$f_0 = 4ksi$$

$$t_{lag} = 8in$$

$$f_v := 60$$
ksi

$$d := t_{lag} - 2in = 6 \cdot in$$

$$d_{bf} := \frac{6}{8} \cdot in$$

$$d_{bf} := \frac{6}{8} \cdot \text{in}$$

$$A_{bf} := \frac{\pi \cdot d_{bf}^2}{4}$$

$$A_{bf} = 0.44 \cdot \text{in}^2$$

$$A_{\rm bf} = 0.44 \cdot {\rm in}^2$$

Spacing :=
$$\frac{(3ft)}{N}$$

$$As := \frac{(12in)}{Spacing} A_{bf} \qquad As = 0.59 \cdot in^2$$

$$As = 0.59 \cdot in^2$$

MOMENT CALCULATION

$$T_{lag} := As \cdot f_y = 35.34 \cdot kips$$

$$a := \frac{T_{lag}}{0.85 \cdot f_r \cdot b} = 0.87 \cdot in$$

$$M := T_{lag} \cdot \left(d - \frac{a}{2} \right) = 196.75 \cdot in \cdot kips$$

$$M_u := M_{\text{max_lag}} \cdot \frac{1.7}{0.9} = 181.04 \cdot \text{in kips}$$

OK!



Project Name: WVARNG South Gate

Project No. _7-7728-0000

Chk'd By:

BALANCED REINFORCEMENT RATIO

$$\rho b \coloneqq \frac{0.85 \cdot \beta_1 \cdot f_c}{f_y} \cdot \frac{87000 \cdot psi}{87000 \cdot psi + f_y}$$

pb = 0.029

Balanced condition

Asmax := $0.75 \cdot \rho b \cdot b \cdot d$ Asmax = $1.54 \cdot in^2$

ACI 10.3.3 "As" provided for flexure must be less than this to ensure ductile behavior <----

$$\rho min := \frac{200 \text{-} psi}{f_y}$$

ACI 10.5 minimum reinforcement required, Alternatively, area of reinforcement provided at every section shall be at least 1/3 greater than that required by analysis. <----

Asmin = $0.24 \cdot \text{in}^2$ Asmin := pmin-b-d

$$a := \frac{As \cdot f_y}{\beta_1 \cdot f_c \cdot b} \qquad a = 0.87 \cdot in$$

$$a = 0.87 \cdot in$$

$$As = 0.59 \cdot in^2$$

check with above Asmax and Asmin criteria

SHEAR CHECK

$$V_n := 2\sqrt{f_c} \cdot b \cdot d = 0.13 \text{ ft}^{0.5} \cdot s \cdot 1b^{-0.5} \cdot \text{kips}$$

$$V_u := V_{\text{max_lag}} \cdot \frac{1.7}{0.85} = 8.71 \cdot \text{kips}$$

OK!

BEARING

$$P_{\text{lag}} := w \cdot \frac{S_{\text{clr}} \cdot 1 \, \text{ft}}{2} = 4.36 \cdot \text{kips}$$

$$A_b := \frac{(P_{\text{lag}} \cdot 1.7)}{(0.7) \cdot 0.85 \cdot f_{*} \cdot 12 \, \text{in}} = 0.26 \cdot \text{in}$$

MIN. BEARING > .260" PER SIDE, OR .347" WITH A 0.75 FACTOR

FOR 3 FEET TALL PANELS, USE 4-#6 BARS SPACED AT 9" HORIZ., WITH 3" CLEAR AND 4-#4 BARS VERTICAL SPACED EQUALLY ACROSS, WITH 3" CLEAR



	WVARNG South Gate
Road Project No. 7-7	728-0000
By: <u>CJR</u>	
	<u>OWD</u>

APPENDIX A SOIL BORING LOGS

amı	eco	AMEC 11003 Bluegrass Parkway Suite 690 Louisville, KY 40299 502-267-0700		•			BOI	RIN	G N		BEI PAGE			
1		ny National Guard	PROJECT											
PROJ	ECT NU		PROJECT LOCATION Camp Dawson, West Virginia GROUND ELEVATION 1280.3 ft HOLE SIZE 3.25											
			GROUND ELEVATION 1280.3 ft HOLE SIZE 3.25											
t		ETHOD HSA	APPENDENCE APPENDITATION											
1		MGS	AT END OF DRILLING											
l	S Dry		AFTER DRILLING											
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i	GRAPHIC LOG	MATERIAL DESCRIPTION		SAMPLE TYPE NUMBER	RECOVERY 9 (RQD)	BLOW COUNTS (N VALUE)	POCKET PEN. (tsf)	DRY UNIT WT. (pcf)	MOISTURE CONTENT (%)	i		PLASTICITÝ INDEX	FINES CONTENT (%)	
		NO SAMPLE POSSIBLE DUE TO PRESENCE OF RIP R	IAP											
5		GRAVEL, FINE, WITH SILTY CLAY, LOOSE		X ss		3-3-4 (7)								
-		ROCK, HIGHLY WEATHERED, SOME SILTY CLAY, DA GRAY		ST 2				100	21	48	28	20	29	
10		CLAY, SILTY, WITH FINE TO COARSE ROCK FRAGMI SOME WEATHERED SHALE, BROWN, STIFF	ENIS.	SS 3		2-3-9 (12)								
STD US LAB.GDT - 12/17/09 08:36 - NAGNTYPROJECTSVARNG SLOPE FAILUREGED.				X ss 4		22-40-47 (87)	7							
A BING		WEATHERED SHALE, LITTLE SILTY CLAY WITH FINE	€ 1 ō	·										
핡		COARSE SAND, DARK GRAY, VERY DENSE SHALE, LIGHT GRAY		SS 5		50/1"	-1							
20 20		17.3-17.5, 17.8-18.0 - HIGHLY WEATHERED ZONES 18.6, 19.3, 21.6, 23.5, 24.5 - SLIGHTLY WEATHERED		FC 6	100 (88)									
907-98:90 90/L/751-		FRACTURE 26.4-26.5, 27.0 - MODERATELY WEATHERED FRACT 27.3 - SLIGHTLY WEATHERED FRACTURE 27.5 - SLIGHT TO MODERATELY WEATHERED FRACTURE 28.3-28.5, 28.7, 28.9, 29.5, 29.6, 30.1, 31.1 - SLIGHTLY WEATHERED FRACTURE	CTURE	RC 7	100 (100)			Andread the Agency Section of the Se						
STD US LAB.GDT -			•	RC 8	92 (50)				· ·					

RC 100 9 (91)

Refusal at 18.3 feet. Bottom of borehole at 33.7 feet.

BORING			
	PAGE		

LOGGED BY MGS

NOTES Dry Hole

AMEC 11003 Bluegrass Parkway Suite 690 Louisville, KY 40299 502-267-0700

CLIENT Army National Guard		
PROJECT NUMBER 7-7728-0000-00		
DATE STARTED 10/9/08	COMPLETED	10/9/08
DRILLING CONTRACTOR MATHES		
DOLLING METHOD HSA		

PROJECT NAME South Gate Road Slope Failure PROJECT LOCATION Gamp Dawson, West Virginia GROUND ELEVATION 1273.7 ft HOLE SIZE 3.25

GROUND WATER LEVELS: AT TIME OF DRILLING ____

AFTER DRILLING ____

AT END OF DRILLING _--

			ш	%		٠,;	·	্ৰ	.IMITS	HG	Z
DEPTH (#)	GRAPHIC	MATERIAL DESCRIPTION	SAMPLE TYPE NUMBER	RECOVERY 9 (RQD)	BLOW COUNTS (N VALUE)	POCKET PEN. (Isf)	DRY UNIT WT. (pcf)	MOISTURE CONTENT (%)	PLASTIC	PLASTICITY INDEX	FINES CONTENT (%)
		GRAVEL, FINE, WITH LITTLE SILTY CLAY AND FINE TO COARSE SAND, LOOSE	X ss		4-4-5 (9)						
5		SILT, CLAYEY, LITTLE FINE TO COARSE SAND, LITTLE FINE GRAVEL, TAN AND GRAY, STIFF	X ss 2		5-6-9 (15)						
		CLAYEY SILT, TAN AND REDDISH BROWN, VERY STIFF	X SS 3		15-9-14 (23)						
10		WEATHERED SHALE, DARK GRAY, VERY STIFF	X SS 4		4-7-10 (17)						
FAILURE GPJ GI		CLAY, SILTY, WITH ROCK FRAGMENTS (HIGHLY WEATHERED), HARD	≓\ ss		50/3"						
₹ <u>15</u>		Defined at 4E 0 foot	1						 -l		1

Refusal at 15.0 feet. Bottom of borehole at 15.0 feet.

an	ne	·C	0	AMEC 11003 Bluegrass Parkway Suite 690 Louisville, KY 40299 502-267-0700					BO	RIN	G N	UM	BE PAGE	1 OF	-5
CLI	ENT	یے ا	<u>Arn</u>					Gate Road							
PRO	IJΕ	CT	NL	IMBER 7-7728-0000-0002 PROJE											
DAT	E S	ST/	\RT	TED 10/9/08 COMPLETED 10/9/08 GROUN	GROUND ELEVATION 1271.2 ft HOLE SIZE 3.25										
DRI	LLI	NG	C	ONTRACTOR MATHES GROUN	D۷	ATER	LEVE	S:							
DRI	LLI	NG	M	E11.00 - 107.	AT TIME OF DRILLING										
LO	3GI	Đ	BY	141335	AT END OF DRILLING										
NO	TES		Dry	Hote A	AFTER DRILLING										
DEPTH	(E)	GRAPHIC	DOG	MATERIAL DESCRIPTION		SAMPLE TYPE NUMBER	RECOVERY % (RQD)	BLOW COUNTS (N VALUE)	POCKET PEN. (tsf)	DRY UNIT WT. (pcf)	MOISTURE CONTENT (%)	LIGUID	IMITS	PLASTICITY INDEX	FINES CONTENT (%)
0				SILT, CLAYEY, MIX OF GRAY AND REDDISH BROWN, MEDIUM STIFF	†	ss 1		4-4-3 (7)			•				
5	-	56 23	7	SILT, CLAYEY, TRACE FINE GRAVEL, LITTLE BLACK NODULES, BROWN, MEDIUM STIFF		(ss		5-4-4 (8)							
1	1		·	SILT, CLAYEY, TRACE BLACK NODULES, TAN, VERY STIFF		SS 3		6-11-11 (22)							
11	0 -					SS 4		8-9-11 (20)				A Community Comm			
SLOPE FAILURE GP3	5	, , , , , , , , , , , , , , , , , , ,		SILT, CLAYEY, LITTLE FINE TO COARSE ROCK FRAGMENTS TAN, VERY STIFF	<u> </u>	SS 5	-	15-12-12 (24)			The state of the s				marini (marantana) and
<u> </u>			Λ.	SHALE, LIGHT GRAY	-	1	-	1							
NAGINT/PROJECTS/A	- 10			18.7, 19.6 - SLIGHTLY WEATHERED FRACTURE 20.0 - SLIGHT TO MODERATELY WEATHERED FRACTURE 20.3-21.2, 21.7-21.9 - ROCK FRAGMENTS, SLIGHT TO MODERATELY WEATHERED 22-23 ZONE OF LOSS 23-23.5 ROCK FRAGMENTS SLIGHT TO MODERATELY		RC 6	100 (72)					***************************************	And the second s		
GINT STD US LAB.GDT - 12/17/08 08-41 - NAGINTPROJEC	25			WEATHERED 23.8 MODERATELY WEATHERED FRACTURE 24.3-24.5 ROCK FRAGMENTS SLIGHT TO MODERATELY WEATHERED 24.8, 24.9 MODERATELY WEATHERED FRACTURE 25.4, 25.8, 25.9, 26.2 - SLIGHT TO MODERATELY WEATHERE FRACTURE 28.1, 29.4, 29.9, 30.3 - SLIGHTLY WEATHERED FRACTURE	D	RC 7	70 (44)								
-GINT STD US L	30			31.1, 31.4 - SLIGHT TO MODERATELY WEATHERED FRACTURE 30.8-31.0 - VERTICAL FRACTURE		RC 8	92 (82)			-			www.mmmdah		

Refusal at 17.0 feet. Bottom of borehole at 32.0 feet.

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8/	ne	ඌ	AMEC 11003 Bluegrass Parkway Suite 690 Louisville, KY 40299 502-267-0700								JME P	SER AGE 1	OF	2
		A		PROJECT	NAME S	South	Gate Road	Slope	Failur	e		<u></u>	, , , , , , , , , , , , , , , , , , ,	-
CL	ENT	AIM) NEW TY		PROJECT	LOCATIO	ON C	amp Daws	on, We	SI VIN	HINE (3 25			
			COMPLETED 10/9/08	GROUND	ELEVATI	ON _1	271 ft	FT	OLE 6)) (*****	2.6.9			_
DE DE	. , 256 i il	NG CO	NTRACTOR MATHES	GROUND	WATER	EVE	.5: :NO							
DE	RILLI	NG ME	THOD HSA				ing <u></u>							
F			MGS		END OF I									
•		Dry		Ar:	1 200			1	T			RBE	iG	F
HLEDTH	(£)	GRAPHIC LOG	MATERIAL DESCRIPTION		SAMPLE TYPE NUMBER	RECOVERY % (ROD)	BLOW COUNTS (N VALUE)	POCKET PEN. (tsf)	DRY UNIT WT. (pcf)	MOISTURE CONTENT (%)	T	PLASTIC W	PLASTICITY	FINES CONTENT (%)
	0		CLAY, SILTY, LITTLE BLACK NODULES, MIX OF TAN, REDDISH BROWN AND GRAY, STIFF		X ss		5-5-6 (11)							
+	5_		SILT, CLAYEY, TRACE FINE GRAVEL, TRACE ORGAI TAN, STIFF		X SS 2	1	5-6-8 (14)	1						
-	•		CLAY, SILTY, WITH FINE GRAINED SAND, REDDISH AND GRAY		SH 3				112	19	39	22	17	91
	10_		CLAY, SILTY, TAN AND REDDISH BROWN, VERY ST		SS 4		10-12-15 (27)							
G SLOPE FAILURE GPJ	- - 15		SILT, CLAYEY, WITH FINE GRAINED SAND, TAN, S	TIFF	X S	3	6-6-6 (12)			15			, and the same of	54
			CLAY, SILTY, SOME FINE ROCK FRAGMENTS, MIX AND DARK GRAY, STIFF	OFTAN	Xs	S 5	8-7-7 (14)			11	0	And the second s	والمراجعة	5
NACINDEOLECTSWAR	2		WEATHERED SHALE, VERY DENSE		<u> </u>	35 7	46-50/ 85	/1-		***************************************	والمستقدية والمستقدية والمستقدية والمستقدية			
. {	ABIG		Market Ma			8 (71)							

RC 93 9 (76)

· (Continued Next Page)

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am	ec®	AMEC 11003 Bluegrass Parkway Sulte 690 Louisville, KY 40299 502-267-0700									• 1	PAGE	2 OF	*
ol ici	AT Am	ny National Guard	PROJECT	NAI	ME_	Śouth	Gate Roa	d Slop	e Failu	ire				
CLIE	IECT N	JMBER 7-7728-0000-0002	PROJECT	LO	CATI	ON C	amp Daw	son, V	est V	rginia			56 T	
DEPTH (ft)		MATERIAL DESCRIPTION		SAMPLE TYPE		RECOVERY % (RQD)	BLOW COUNTS (N VALUE)	POCKET PEN. ((sf)	DRY UNIT WT. (pof)	MOISTURE CONTENT (%)	CINGID	PLASTIC WEBE	PLASTICITY &	FINES CONTENT (%)
35		SHALE, LIGHT GRAY			10	100 (85)								
		28.4 - MODERATELY TO HIGHLY WEATHERED FRAC 28.9 - SLIGHTLY WEATHERED FRACTURE 29.2, 29.4 - MODERATELY WEATHERED FRACTURE 29.8-30.3 - VERTICAL FRACTURE, SLIGHTLY WEATH		H										·
+		30.5 - MODERATELY WEATHERED TRACTURE			RC 11	100 (61)								
40		30.5-30.7 - VERTICAL FHACTURE 31.8 - SLIGHTLY WEATHERED FRACTURE 33.0 - SLIGHTLY WEATHERED FRACTURE		LL	<u></u>	<u> </u>	<u> </u>	<u> </u>	1	<u>.l</u>	1	1	.L	
		35.2, 36.8-36.9, 37.2, 38.2 - SLIGHTLY WEATHERED												
		38.9 - MODERATELY WEATHERED FRACTURE 39.3 - SLIGHT TO MODERATELY WEATHERED 40.1, 40.7 - SLIGHTLY WEATHERED FRACTURE (con	ntinued											
		from previous page)												
		Bottom of borehole at 40.9 feet.												1 1
2														
PEFAI														
0 810														
SARIN	,			٠										
OJECT														
HACE														
<u>5</u>		•												
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ECH BH COLUMNS - GINT STD US LAB.GDT - (27/7/08 08:45 - M/GINTPROJECTS/ARNG SLOPE FAILURE.GBU														
<u>F</u>		dis kan kiringan ar ini. Tara daga kabang dang sa panggan kiring ananggan sa tara sa daga mananda panggan pang	のうまとは 7年14首 H bastel (perse) 1000						Addition to the com-					
ECH B														

S02-287-7000 LIENT Army National Quard ROJECT NUMBER 7-7728-0000-0002 PROJECT COATION Camp Devison, West Virginia ROJECT NUMBER 7-7728-0000-0002 ATE STARTED 10/8/08 COMPLETED 10/8/08 GROUND ELEVATION 1286 II HOLE SIZE 3.25 RILLING CONTRACTOR MATHES RILLING CONTRACTOR MATHES RILLING CONTRACTOR MATHES AT TIME OF PRILLING AT THE OF PRILLING AT THE OF PRILLING AT THE OF PRILLING AT THE OF PRILLING ATTER DRILLING	ENT Army National Guard DIECT NUMBER 7-7728-0000-0002 PROJECT NUMBER 7-7728-0000-0002 PROJECT LOCATION Camp Dawson, West Virginia GROUND ELEVATION 1266 ft HOLE SIZE 3.25 GROUND WATER LEVELS: AT TIME OF DRILLING ATERNOOPHING MATHES ILLING CONTRACTOR MATHES ILLING METHOD HSA GRED BY MGS MATERIAL DESCRIPTION CLAY, SILTY, SOME FINE TO COARSE SAND, TRACE FINE GRAVEL, BROWN, MEDIUM STIFF CLAY, SILTY, SOME FINE TO COARSE SAND, BROWN, VERY STIFF CLAY, SILTY, SOME FINE TO COARSE SAND, BROWN, VERY STIFF CLAY, SILTY, SOME FINE TO COARSE SAND, BROWN, VERY STIFF CLAY, SILTY, SOME FINE TO COARSE SAND, BROWN, VERY STIFF CLAY, SILTY, SOME FINE TO COARSE SAND, BROWN, VERY STIFF CLAY, SILTY, SOME FINE TO COARSE SAND, BROWN, VERY STIFF CLAY, SILTY, SOME FINE TO COARSE SAND, BROWN, VERY STIFF CLAY, SILTY, SOME FINE TO COARSE SAND, BROWN, VERY STIFF CLAY, SILTY, SOME FINE TO COARSE SAND, BROWN, VERY STIFF CLAY, SILTY, SOME FINE TO COARSE SAND, BROWN, VERY STIFF CLAY, SILTY, SOME FINE TO COARSE SAND, BROWN, STIFF SILT, CLAYEY, TRACE FINE TO COARSE ROCK FRAGMENTS, SILTY, SOME FINE TO COARSE ROCK FRAGMENTS, SILTY, SOME FINE TO COARSE ROCK FRAGMENTS, SILTY, SOME FINE TO COARSE SAND, BROWN, STIFF SILT, CLAYEY, TRACE FINE TO COARSE ROCK FRAGMENTS, SILTY, SOME FINE TO COARSE SAND, BROWN, STIFF SILT, CLAYEY, TRACE FINE TO COARSE ROCK FRAGMENTS, SILTY, SOME FINE TO COARSE ROCK FRAGME		AMEC 11003 Bluegrass Parkway Suite 690 Louisville, KY 40299		-			BOF	3IN	G N	UM	BEF AGE	1 OF	7
THENT Army National Claid Army National Claid ROJECT LOCATION Camp Dawson, West Virginia ROJECT LOCATION GROUND ELEVATION 10808 COMPLETED 10/8/08 GROUND ELEVATION 1266 ft HOLE SIZE 3.25 GROUND WATER LEVELS: AT TIME OF DRILLING ATTERD PRILLING ATTERDERG LIMITS ATTERDERG LIMI	DISCOUNDER 7-7728-0000-0002 DISCOUNDER 7-7728-0000-0002 TE STARTED 10/8/08 COMPLETED 10/8/08 GROUND ELEVATION 1266 ft HOLE SIZE 3.25 GROUND WATER LEVELS: AT TIME OF DRILLING AT END OF DRI		502-267-0700	DOA IECT	NAUF	South	Gate Road	Slope	. Fallu	re				
ATTESTARTED 10/8/08 COMPLETED 10/8/08 GROUND ELEVATION 1266 ft HOLE SIZE 3.25 RILLING CONTRACTOR MATHES RILLING METHOD HSA OGGED BY MGS OTES DIY Hole MATERIAL DESCRIPTION OCIAY, SILTY, SOME FINE TO COARSE SAND, TRACE FINE GRAVEL, BROWN, MEDIUM STIFF CLAY, SILTY, SOME FINE TO COARSE SAND, BROWN, VERY STIFF CLAY, SILTY, SOME FINE TO COARSE SAND, BROWN, VERY STIFF CLAY, SILTY, SOME FINE TO COARSE SAND, BROWN, VERY STIFF CLAY, SILTY, SOME FINE TO COARSE SAND, BROWN, VERY STIFF CLAY, SILTY, SOME FINE TO COARSE SAND, BROWN, VERY STIFF CLAY, SILTY, SOME FINE TO COARSE SAND, BROWN, VERY STIFF CLAY, SILTY, SOME FINE TO COARSE SAND, BROWN, VERY STIFF CLAY, SILTY, SOME FINE TO COARSE SAND, BROWN, VERY STIFF CLAY, SILTY, SOME FINE TO COARSE SAND, BROWN, VERY STIFF SILT, CLAYER, TRACE FINE TO COARSE ROCK FRAGMENTS, BROWN, HARD 15 16 17 18 19 19 10 10 11 11 12 13 14 15 15 15 15 15	CLAY, SILTY, SOME FINE TO COARSE SAND, TRACE FINE GRAVEL, BROWN, MEDIUM STIFF CLAY, SILTY, SOME FINE TO COARSE SAND, TRACE FINE GRAVEL, BROWN, MEDIUM STIFF CLAY, SILTY, SOME FINE TO COARSE SAND, BROWN, VERY STIFF CLAY, SILTY, SOME FINE TO COARSE SAND, BROWN, VERY STIFF CLAY, SILTY, SOME FINE TO COARSE SAND, BROWN, VERY STIFF CLAY, SILTY, SOME FINE TO COARSE SAND, BROWN, VERY STIFF CLAY, SILTY, SOME FINE TO COARSE SAND, BROWN, STIFF SILT, CLAYEY, TRACE FINE TO COARSE ROCK FRAGMENTS, BROWN, HARD SILT, CLAYEY, TRACE FINE TO COARSE ROCK FRAGMENTS, BROWN, HARD SILT, CLAYEY, TRACE FINE TO COARSE ROCK FRAGMENTS, BROWN, HARD SILT, CLAYEY, TRACE FINE TO COARSE ROCK FRAGMENTS, BROWN, HARD SILT, CLAYEY, TRACE FINE TO COARSE ROCK FRAGMENTS, BROWN, HARD	ENT Army	National Guard	PROJECT	LOCATI	ON C	amp Daws	on, W	est Vi	rginia				
ATE STATELLING MATHES GROUND WATER LEVELS: AT TIME OF DRILLING AT END OF DRILLING AT END OF DRILLING ATTERBERG LIMITS AT	TE STATED TWO THE STIFF TE STATED TWO THE STIFF TE STATED TWO THE STIFF THE OF DRILLING	JECT NUI	WBER 7-7728-0000-0002		ELEVAT	ION _1	266 ft		OLE	SIZE	3.25			
AT TIME OF DRILLING	ILLING METHOD HSA AT END OF DRILLING GRED BY MGS AFTER DRILLING AFTER DRILLING AFTER DRILLING AFTER DRILLING AFTER DRILLING ATTERBERG LIMITS ATTERBOWN ATTERBOW	E STARTE	D 10/8/08 COMPLETED 18/8/08											
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AFTER DRILLING MATERIAL DESCRIPTION MATERI	THE DITY Hole MATERIAL DESCRIPTION MATERIA													-
MATERIAL DESCRIPTION	MATERIAL DESCRIPTION WALL SILTY, SOME FINE TO COARSE SAND, TRACE FINE GRAVEL, BROWN, MEDIUM STIFF CLAY, SILTY, SOME FINE TO COARSE SAND, BROWN, VERY STIFF CLAY, SILTY, SOME FINE TO COARSE SAND, BROWN, VERY STIFF CLAY, SILTY, SOME FINE TO COARSE SAND, BROWN, STIFF SILT, CLAYEY, TRACE FINE TO COARSE ROCK FRAGMENTS, BROWN, HARD SILT, CLAYEY, TRACE FINE TO COARSE ROCK FRAGMENTS, BROWN, HARD SILT, CLAYEY, TRACE FINE TO COARSE ROCK FRAGMENTS, BROWN, HARD SILT, CLAYEY, TRACE FINE TO COARSE ROCK FRAGMENTS, BROWN, HARD SS. 6-8-28 (36)			. AF	TER DRIL	LING		7			ATT	ERBE	RG T	 }
CLAY, SILTY, SOME FINE TO COARSE SAND, TRACE FINE GRAVEL, BROWN, MEDIUM STIFF CLAY, SILTY, SOME FINE TO COARSE SAND, TRACE FINE GRAVEL, BROWN, MEDIUM STIFF CLAY, SILTY, SOME FINE TO COARSE SAND, BROWN, VERY STIFF CLAY, SILTY, SOME FINE TO COARSE SAND, BROWN, VERY STIFF CLAY, SILTY, SOME FINE TO COARSE SAND, BROWN, STIFF SILT, CLAYEY, TRACE FINE TO COARSE ROCK FRAGMENTS, BROWN, HARD SILT, CLAYEY, TRACE FINE TO COARSE ROCK FRAGMENTS, BROWN, HARD SS 6-8-28 (36)	CLAY, SILTY, SOME FINE TO COARSE SAND, TRACE FINE GRAVEL, BROWN, MEDIUM STIFF CLAY, SILTY, SOME FINE TO COARSE SAND, TRACE FINE GRAVEL, BROWN, MEDIUM STIFF CLAY, SILTY, SOME FINE TO COARSE SAND, BROWN, VERY STIFF CLAY, SILTY, SOME FINE TO COARSE SAND, BROWN, VERY STIFF CLAY, SILTY, SOME FINE TO COARSE SAND, BROWN, STIFF SILT, CLAYEY, TRACE FINE TO COARSE ROCK FRAGMENTS, BROWN, HARD SILT, CLAYEY, TRACE FINE TO COARSE ROCK FRAGMENTS, BROWN, HARD SS 6-8-28 (36)	T			PLE TYPE JMBER	OVERY % (RQD)	BLOW OUNTS VALUE)	KET PEN. (tsf)	(pd)	DISTURE ATENT (%)	!	IMITS	TICITY DEX	S CONTEN
CLAY, SILTY, SOME FINE TO COARSE SAND, TRACE FINE GRAVEL, BROWN, MEDIUM STIFF CLAY, SILTY, SOME FINE TO COARSE SAND, BROWN, VERY STIFF CLAY, SILTY, SOME FINE TO COARSE SAND, BROWN, STIFF CLAY, SILTY, SOME FINE TO COARSE SAND, BROWN, STIFF SILT, CLAYEY, TRACE FINE TO COARSE ROCK FRAGMENTS, BROWN, HARD SS (36) SS (36) SS (36)	CLAY, SILTY, SOME FINE TO COARSE SAND, TRACE FINE GRAVEL, BROWN, MEDIUM STIFF CLAY, SILTY, SOME FINE TO COARSE SAND, BROWN, VERY STIFF CLAY, SILTY, SOME FINE TO COARSE SAND, BROWN, STIFF CLAY, SILTY, SOME FINE TO COARSE SAND, BROWN, STIFF SILT, CLAYEY, TRACE FINE TO COARSE ROCK FRAGMENTS, BROWN, HARD SILT, CLAYEY, TRACE FINE TO COARSE ROCK FRAGMENTS, BROWN, HARD SS 3-3-3 (6) 4-7-8 (15) SS 4 4-7-8 (15) SS 4 6-8-28 (36)] [SAM	REO	_0g	ğ	YHO	₹g	==	53	Ž≅	
GRAVEL, BHOWN, MEDIUM STIFF CLAY, SILTY, SOME FINE TO COARSE SAND, BROWN, VERY STIFF CLAY, SILTY, SOME FINE TO COARSE SAND, BROWN, STIFF CLAY, SILTY, SOME FINE TO COARSE SAND, BROWN, STIFF SS 4 4-7-8 (15) 15 SILT, CLAYEY, TRACE FINE TO COARSE ROCK FRAGMENTS, BROWN, HARD SILT, CLAYEY, TRACE FINE TO COARSE ROCK FRAGMENTS, BROWN, HARD SS 3 6-8-28 (36)	GRAVEL, BHOWN, MEDIUM STIFF CLAY, SILTY, SOME FINE TO COARSE SAND, BROWN, VERY STIFF CLAY, SILTY, SOME FINE TO COARSE SAND, BROWN, STIFF CLAY, SILTY, SOME FINE TO COARSE SAND, BROWN, STIFF SILT, CLAYEY, TRACE FINE TO COARSE ROCK FRAGMENTS, BROWN, HARD SILT, CLAYEY, TRACE FINE TO COARSE ROCK FRAGMENTS, BROWN, HARD SS 6-8-28 (36)													
CLAY, SILTY, SOME FINE TO COARSE SAND, BROWN, STIFF CLAY, SILTY, SOME FINE TO COARSE ROCK FRAGMENTS, BROWN, HARD 15 16 17 18 18 18 18 18 18 18 18 18	CLAY, SILTY, SOME FINE TO COARSE SAND, BROWN, STIFF CLAY, SILTY, SOME FINE TO COARSE ROCK FRAGMENTS, BROWN, HARD SILT, CLAYEY, TRACE FINE TO COARSE ROCK FRAGMENTS, BROWN, HARD SS 6-8-28 (36)	5	GRAVEL, BROWN, MEDIUM STIFF		<u> </u>					The state of the s				
10 V SILT, CLAYEY, TRACE FINE TO COARSE ROCK FRAGMENTS, BROWN, HARD SS 6-8-28 (36)	10 SILT, CLAYEY, TRACE FINE TO COARSE ROCK FRAGMENTS, BROWN, HARD 15 • 4 SILT, CLAYEY, TRACE FINE TO COARSE ROCK FRAGMENTS, SS 6-8-28 (36)		STIFF		M 3					15		ŗ		5
BROWN, HARD SS 6-8-28 (36)	SS 6-8-28 (36)	10			N 4					15				6
		-, G	SILT, CLAYEY, TRACE FINE TO COARSE ROCK FR BROWN, HARD	RAGMENTS	M SS									
20	Refusal at 21.0 feet. Bottom of borehole at 21.0 feet.	\ \T\	SILT, CLAYEY, DARK GRAY, STIFF	,			,							
Refusal at 21.0 feet. Bottom of borehole at 21.0 feet.	Refusal at 21.0 feet. Bottom of borehole at 21.0 feet.	20			X s	S				1	5			
Bollow St. 25. Company			Refusal at 21.0 feet. Bottom of borehole at 21.0 feet.											
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am	ec	AMEC 11003 Bluegrass Parkway Suite 690 Louisville, KY 40299 502-267-0700	19-0-19-0-19-0-19-0-19-0-19-0-19-0-19-0		·	<u> </u>	BOI	RIN	G N			R E	
מפמ	ICCT A	my National Guard	PROJEC	T LOCATI	ON C	Gate Road	son, W	est Vi	ginia				
DRIL.	LING (RTED 10/8/08 COMPLETED 10/8/08 CONTRACTOR MATHES METHOD HSA	GROUND	WATER	LEVEL	<u>1266.4 ft</u> .s: .ing							
LOG	GED B	BY MGS By Hole	TA		DRILL	ING							
DEPTH	0		j	SAMPLE TYPE NUMBER	RECOVERY % (RQD)	BLOW COUNTS (N VALUE)	POCKET PEN. (tsf)	DRY UNIT WT. (pcf)	MOISTURE CONTENT (%)		PLASTIC WINT	PLASTICITY 3	FINES CONTENT (%)
- -		CLAY, SILTY, LITTLE FINE TO COARSE GRAVE BROWN AND GRAY, VERY STIFF	EL, REDDISH	X ss		8-9-11 (20)			-				
5		CLAY, SILTY, TRACE FINE GRAVEL, TAN, STIFF		X ss 2		9-5-7 (12)							
1		CLAY, SILTY, LITTLE ORGANICS, LITTLE FINE T SAND, TRACE FINE GRAVEL, MIX OF REDDISH GRAY, STIFF CLAY, SILTY, LITTLE FINE TO COARSE SAND, I		SS 3 VI SS		3-6-6 (12) 3-5-5	_						
10	-	NODULES, MIX OF GRAY AND BROWN, STIFF	na, nagas jumpi poljet žings; binšei balla	X 4		(10)							
15		CLAY, SILTY, SOME FINE TO COARSE ROCK F LITTLE BLACK NODULES, BROWN AND REDDI MEDIUM STIFF	RAGMENTS, SH BROWN,	X ss 5		4-3-3 (6)				and the second s			
200000000000000000000000000000000000000		SILT, CLAYEY, BROWN AND REDDISH BROWN	I, STIFF	X SS		3-4-6 (10)			16				
8 08:45 · N:(5)		SHALE				EOM*							
INT STD US LAB.GDT - 12/17/06 08:45 - NAGINIVPROJECTS		Refusal at 23.5 feet. Bottom of borehole at 23.5 feet.		\ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \	Ľ	50/1"							

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mec [©]	AMEC 11003 Bluegrass Parkway Suite 690 Louisville, KY 40299 502-267-0700	•							PAGE		
LIENT ATT		IECT NAME _S						······································	,	··········	
	PROJ	ECT LOCATIO							***************************************		
TE STAR	TED 10/8/08 COMPLETED 10/8/08 GROI	UND ELEVATION			[}]	OLE	SIZE .	3.25	·····		
RILLING C	ONTRACTOR MATHES GROU	UND WATER L	EVEL	S:							
RILLING M	ETHOD HSA	AT TIME OF I	MILL	NG							
GGED BY	MGS	AT END OF D								***************************************	
OTES _Dn	y Hole	AFTER DRILL	JNG					ATT	ERBE	PO I	*************
GRAPHIC LOG	MATERIAL DESCRIPTION	SAMPLE TYPE NUMBER	RECOVERY % (ROD)	BLOW COUNTS (N VALUE)	POCKET PEN. (Isf)	DRY UNIT WT. (pcf)	MOISTURE CONTENT (%)	CIMIT	IMITS	PLASTICITY NOEX	FINES CONTENT
	SILT, CLAYEY, WITH FINE TO COARSE GRAVEL, SOME BLACK NODULE, BROWN, VERY STIFF	X SS		6-9-14 (23)					-		
5	SAND, SILTY, THACE FINE TO COARSE SAND, TRACE FIN GRAVEL, TRACE ORGANICS, BROWN, STIFF	E SS 2		6-6-8 (14)			16				47
	CLAY, SILTY, LITTLE FINE TO COARSE SAND, TRACE BLACK NODULE, GRAY, MEDIUM STIFF	X SS 3		4-2-3 (5)	-		15	38	23	15	
10	SILT, CLAYEY, TRACE FINE GRAVEL, BROWN, MEDIUM STIFF	SS 4		3-3-4 (7)	-						
15	CLAY, SILTY, SOME FINE TO COARSE SAND, LITTLE BLAG NODULES, BROWN, STIFF	SS 5		2-4-6 (10)							
	SILT, CLAYEY, LITTLE WEATHERED SHALE, BROWN AND TAN, VERY STIFF	o V ss		6-8-10	_			The second secon			
20		X 6		(18)			***************************************				
25	SILT, CLAYEY, WITH FINE GRAINED SAND, REDDISH BROWN, MEDIUM STIFF	X ss 7		3-2-4 (6)					***************************************		
30		RC 8	97 (20)								
*	AND THE PROPERTY OF THE PROPER		91	<u></u>	,			_			

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		7 100101121				Gate Roa Camp Daw							
PROJ	ECT NU	MBER 7-7728-0000-0002							[ATT	ERBE	RG	<u> </u>
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5		CLAY, SILTY, SOME FINE TO COARSE SAND, BROWN AND TAN, SOFT	5	X SS		1-2-2 (4)							
10		CLAY, SILTY, SOME FINE TO MEDIUM GRAINED SAND, BROWN AND GRAY, SOFT	,,,, sano bee	SS 4		1-1-2 (3)							
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5		CLAY, SILTY, WITH FINE TO COARSE GRAINED SAND AND BROWN, MEDIUM STIFF (PROBABLE FILL)		SS 2	·	3-3-4 (7)							
		CLAY, SILTY, WITH FINE TO COARSE SAND, LITTLE F GRAVEL, BROWN, MEDIUM STIFF (PROBABLE FILL)	INE	SS 3		3-2-3 (5)	-						
10		CLAY, SILTY, LITTLE FINE TO COARSE SAND, SOME ORGANICS, ORGANIC ODOR, DARK GRAY, SOFT		X SS 4		2-2-2 (4)	_						
		CLAY, SILTY, LITTLE FINE TO COARSE SAND, REDDIN BROWN, STIFF	3H	X ss 5		4-4-5 (9)			20	41	23	18	1
15		CLAY, SILTY, SOME FINE TO COARSE SAND, SOME I	BLACK										
20		NODULES, TAN, VERY STIFF		X SS		5-7-9 (16)	_						
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		Refusal at 27.0 feet. Bottom of borehole at 27.0 feet.											
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0	90	SILT, CLAYEY, SOME FINE TO COARSE GRAVEL, LIT FINE TO COARSE SAND, TAN, STIFF, DRY		X ss	,	6-7-6 (13)							
5		SILT, CLAYEY , SOME FINE TO COARSE SAND, TAN, N STIFF		X SS 2		6-5-6 (11)	1	***************************************				La proprieta de la compansa de la co	
		CLAY, SILTY, SOME FINE TO COARSE GRAINED SAND MOIST, STIFF), TAN,	X ss 3		6-6-5 (11)							
10	SICILA	NO RECOVERY		SS 4		8-4-3 (7)							
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20		CLAY, VERY SILTY, REDDISH BROWN, SOME FINE T COARSE GRAINED SAND, LITTLE FINE TO COARSE F FRAGMENTS, MOIST, STIFF	O ROCK	X ss 6		4-5-5 (10)				***************************************	A THE REAL PROPERTY OF THE PRO		
25		CLAY, SILTY, BROWN AND TAN, SOME FINE TO COA SAND, LITTLE BLACK NODULES, MOIST, HARD	VASE -	SS 7		9-15-16 (31)	3		Administration of the second o				
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Y	MILS (mm)
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С	109 (2.7 mm)
D	138 (3.5 mm)
Е	168 (4.3 mm)
F	188 (4.8 mm)
G	218 (5.5 mm)
Н	249 (6.3 mm)
J	280 (7.1 mm)

613-BLANK

SECTION 614 PILING WALLS

614.1 - DESCRIPTION:

This work shall consist of furnishing and placing steel piles in predrilled holes, concrete or grout, backfill and lagging, of the kinds and dimensions designated, in accordance with these provisions and in reasonably close conformity with the lines, grades, dimensions, and locations shown on the Plans or established by the Engineer. Painting of the exposed steel is included.

Careful attention shall be given to assuring the pile wall will tie directly into an existing stable slope. Prior to ordering any materials, the contractor in conjunction with the Engineer shall conduct a project site review in order to verify the limits of the pile wall.

614.2 - MATERIALS:

Materials shall conform to the requirements specified in the following Subsections of Division 700:

MATERIAL	SUBSECTION
Steel Piles and Splices	709.12
Steel Lagging and Wales	709.12
Reinforcing Steel	709.1
Prestressing Steel	709.2
Treated Timber Lagging	710
Portland Cement	701.1
Fine Aggregate	702.1
Fly Ash	707.4

614.3 - DRILLING:

614.4

A drilled hole is required for the buried length of the pile.

A minimum of 1/3 the total pile length or 10 feet (3 m), whichever is greater, is to be placed in bedrock/shale. Deviation from this requirement will be controlled by a Plan note. The total estimated pile length and the depth to the estimated bedrock/shale line are shown on the piling profile. Should the elevation of the actual bedrock/shale vary from the estimated elevation by more than 2.5 feet (0.8 m), the Engineer must approve the hole prior to placement of the pile. The material from the drilled hole shall be removed and disposed of by the Contractor in an approved site.

Particular care must be taken in the drilling operation to avoid deflecting the bit along a sloping bedrock/shale line. To verify proper alignment, the Contractor shall measure and record the vertical alignment of the hole using a plumb bob or other acceptable method.

Preferably, the diameter of the drilled hole shall be a size that will allow the pile, while being slowly lowered into the hole, to reach the bottom of the hole under the impetus of the pile weight. The minimum hole diameter shall be 2 inches (50 mm) larger than the diagonal distance across the pile cross section.

Light tapping (ten blows with at least 3 inches (75 mm) of penetration per blow) with a pile hammer exerting no more than 12,000 ft/lbs (16 kJ) of energy is permitted at the direction of the Engineer to advance the pile past minor obstacles in the hole.

Temporary casing of holes may be needed to maintain an open clean hole through the soil overburden. There will be no additional compensation for temporary casing. The cost of any casing used shall be included in the unit price bid for piling.

614.4 - INSTALLATION OF PILES:

Piles shall be located as shown on the Plans or as directed by the Engineer. Piles shall be installed with the pile center within 1 inch (25 mm) of the Plan location. The piles must be prevented from rotating, so that the pile axis is within five degrees of the position shown on the Plans.

The maximum permissible vertical deviation for piles shall be one percent of the total pile length, as measured at the actual pile location.

It is desirable that piles be installed without splicing; however, at the direction of the Engineer splices may be made. Splice lengths at the top of the piles may be butt welded provided the splice lengths are less than the required splice plates. No payment will be made for cut-offs. Welding shall be in accordance with 615.3.16.

Accurate records shall be maintained by the Contractor showing the depth to which each pile was placed, the plumbness, the amount of material used, elevation of bedrock/shale, and any unusual conditions encountered during the pile installation. These records shall be incorporated into the permanent records of the project.

614.5 - CORROSION PROTECTION:

Piles will be protected from corrosion and sealed by the placement of

concrete or grout, from the bottom of the hole to the bottom of the lagging or as directed by the Engineer. Vibration of the concrete or grout is not required. The Contractor shall complete all concrete or grout operations for holes drilled during the work day.

The drilled hole shall be pumped free of water and shall be reasonably free of fall-in soil or other debris prior to the placement of the concrete or grout. The concrete or grout in the bedrock/shale portion of the hole will be pumped or tremied through a pipe beginning at the bottom of the drilled hole. The pipe shall be slowly raised ensuring the pipe end remains at least 2 feet (600 mm) below the surface of the concrete or grout. A means of positively measuring the elevation of the concrete or grout as it is placed shall be provided by the Contractor.

After placing the concrete or grout in the bedrock/shale, the Contractor has the option of either pumping or pouring directly into the hole the remainder of the concrete or grout. Placing the concrete or grout from the bottom of the hole to the bottom of the lagging shall be accomplished in one continuous operation.

The Contractor will inform the Engineer, at the preconstruction conference, as to the type of corrosion protection that will be used. Intermixing of concrete and grout will not be allowed, unless approved by the Engineer.

Concrete shall be in accordance with Section 601, Class B. The job site testing is waived

Grout will be furnished and placed in accordance with the requirements specified herein.

The acceptance sampling and testing of the grout is the responsibility of the Division.

Quality Control of the concrete or grout is there responsibility of the Contractor as designated in Materials Procedure MP 601.03.50. The Contractor shall maintain equipment and qualified personnel, who shall direct all field inspection, sampling, and testing necessary to determine the magnitude of the various properties of the concrete and grout governed by the Specifications and shall maintain these properties within the limits of this Specification. The Quality Control Plan designated in MP 601.03.50 shall be submitted to the Engineer at the pre construction conference. Work shall not begin until the Plan is reviewed for conformance with the contract documents.

The required 7-day compression strength of the grout shall be a minimum of 2,000 psi (14 MPa). Grout which does not attain the 2,000 psi (14 MPa) strength in 7 days but exceeds a strength of 1,600 psi (11 MPa) shall be subject to price reduction based on the percentage of strength attained.

A grout strength test shall consist of testing three 6 in \times 12 in (150 mm \times 300 mm) cylindrical specimens. The test results shall be the average of the three specimens. One set of three specimens shall be made for each day's operations.

The bid price for the piling with grout compressive strengths greater than or equal to 2,000 psi (14 MPa) will be paid at 100 percent unless the piling installation does not meet Specifications for other reasons. Between 1,600 psi

614.6

(11 MPa) and 2,000 psi (14 MPa) compressive strengths, the cost of the grout will be deducted from the actual grout cost on a proportional basis with 2,000 psi (14 MPa) being 100 percent and 1,600 psi (11 MPa) being zero percent payment. With 1,600 psi (11 MPa) grout, the piling installation would be considered to meet 80 percent of the Specifications and the penalty being zero payment for the grout.

The penalty would involve only the quantity of grout represented by the failing compressive strength results.

The bid price for the piling will be reduced for the piles grouted with grout having less than 1,600 psi (11 MPa) compressive strengths as follows:

A = Compressive strength of grout

B = Total foot (meter) of piling grouted with

C = Unit bid price per foot (meter) of piling

D = Cost of grout (from Contractor)

E = 2,000 psi (14 Mpa)

F = Total penalty

F = D + [BC - D] 10.80 - (A + E)

614.6 - PAINTING:

All surfaces from the top of the steel pile, down to and including 2.0 ft. (600 mm) below the top of the anticipated grout line shall be cleaned and painted. The method of surface preparation shall be hand tool cleaning to SSPC-SP-2. The paint system shall consist of one-coat of aluminum epoxy mastic meeting the requirements of 711.12 applied at a minimum dry film thickness of 5 mils (125 μ m).

614.7 - LAGGING AND BACKFILLING:

Lagging of the type and size as specified on the Plans shall be installed between the piles. Backfilling and restoration of the roadway template shall be as shown on the Plans.

Timber lagging shall be Grade #3 or better treated rough cut oak, 3 in (75 mm) wide by 8 in (200 mm) deep for heights up to 11 ft (3.4 m); and for wall heights exceeding 11 ft (3.4 m) the timber lagging shall be double 3 in (75 mm) wide by 8 in (200 mm) deep. The boards shall be cut to their required length prior to preservative treatment.

The timber lagging shall conform to Sections 710.3 and 710.4 of the West Virginia Division of Highways Standard Specifications and shall be CCA treated for soil and fresh water use, as per AWPA C2.

614.8 - METHOD OF MEASUREMENT:

The quantity of piles will be measured in linear feet (meters) of piles installed and accepted for the wall. The quantity of lagging will be measured in square feet (meters) of lagging installed and accepted for the wall.

614.9-BASIS OF PAYMENT:

The quantities will be paid for at the contract unit prices bid for the items listed below, which prices and payments shall be full compensation for furnishing all materials and doing all the work herein prescribed in a workmanlike and acceptable manner, including all labor, tools, equipment, supplies, and incidentals necessary to complete the work. The cost of drilling, concrete, grout, wales, and painting shall be included in the price bid for the piles. The cost of painting and welding steel lagging shall be included in the price bid for steel lagging.

614.10-PAY ITEMS:

ITEM	DESCRIPTION	UNIT
614001-*	"size" STEEL PILE	LINEAR FOOT (METER)
614002-*	STEEL LAGGING, THICKNESS "thickness"	SQUARE FOOT (METER)
614003-*	CONCRETE LAGGING, THICKNESS "thickness"	SQUARE FOOT (METER)
614004-*	TIMBER LAGGING	SQUARE FOOT (METER)

^{*} Sequence number



SUPPLEMENTARY SPECIFICATIONS

The specifications for project shall be the West Virginia Department of Highways, "Standard Specifications, Roadways and Bridges", except as modified herein.

614.1 - DESCRIPTION

Delete the last sentence in the first paragraph

614.3 - DRILLING

Revise the second paragraph to read the following:

"The minimum embedment of the pile length into bedrock shall be designated on the plans. The total estimated pile length and depth to estimated bedrock are shown in the 'Drilled Shaft Schedule' and on the 'Drilled Shaft Profile'. Should the actual elevation vary by more than 2.5 feet, the Engineer must approve the hole prior to placement of the pile. The material from the drilled hole shall be removed and disposed of by the Contractor in an approved site."

Revise portions of the fourth paragraph to state the minimum diameter of the drilled hole shall be as shown on the plans.

614.5 - CORROSION PROTECTION

Revise the first paragraph to indicate that vibration of concrete will be required for the upper ten feet of the drilled shaft.

Delete from paragraph 11 to the end of section 614.5 inclusively. All concrete is expected to reach a minimum 7 day strength of 1,600 psi and 28 day strength of 4,000 psi.

614.6 - PAINTING

Delete this section in its entirety. Painting of the structural steel is not required.

614.7 - LAGGING AND BACKFILLING

Delete paragraphs two and three in this section. Timber lagging is not applicable to this project.

MEASUREMENT AND PAYMENT

LINE ITEM 1 - Mobilization and Demobilization

This item will cover the payment for the mobilization and demobilization of all plant and equipment to execute the project. Payment will be made on a LUMP SUM basis.

LINE ITEM 2 - Clear & Grub

This item will cover the payment for clearing and grubbing the area of work, of all trees, shrubs, etc. Payment will be made on a PER ACRE basis.

LINE ITEM 3 – Excavation and Embankment

This item will cover payment for all excavation and embankment not otherwise included in any other line items on the Bid Form. These items include, but are not limited to, excavation and grading in front of the drilled shaft wall, excavation for the precast concrete lagging, any and all benching that may be required, miscellaneous backfill that may be required, etc. Payment will be made on a LUMP SUM basis.

LINE ITEM 4 – Erosion & Sediment Control

This item will cover all erosion and sediment control measures incorporated by the Contractor's approved SWPPP. Payment will be made on a LUMP SUM basis.

LINE ITEM 5 - Roadway Grading

This item will cover the scarifiying, mixing and recompacting of the upper 12" of the existing roadbed. Payment for this item will be made on a per CUBIC YARD basis.

LINE ITEM 6 - Stone Base for Road

This item will cover the 12" of ABC crushed stone to be placed for the new roadway. Payment for this item will be made on a per TON basis.

LINE ITEM 7 – 30" Diameter Drilled Shafts, Above Bedrock

This item will cover all means, methods and materials to perform the machine excavation of the material above bedrock at the drilled shaft locations. Also included in this item shall be the concrete fill material, spoil removal, and forming above grade if necessary. Payment for this item will be made on a per LINEAR FOOT basis.

LINE ITEM 8 - 30" Diameter Drilled Shafts, Into Bedrock

This item will cover all means, methods and materials to perform the machine excavation of the material into bedrock at the drilled shaft locations. Also included in this item shall be the concrete fill material and spoil removal. Payment for this item will be made on a per LINEAR FOOT basis.

LINE ITEM 9 – Steel Piles, W18x106

This item will cover all means, methods and materials to furnish, fabricate and place the steel piles at the proper centerline locations indicated. Payment for this item will be made on a per LINEAR FOOT basis.

LINE ITEM 10 - Steel Piles, W21x111

This item will cover all means, methods and materials to furnish, fabricate and place the steel piles at the proper centerline locations indicated. Payment for this item will be made on a per LINEAR FOOT basis.

LINE ITEM 11 - Precast Concrete Lagging, 8" Thick

This item will cover all means, methods and materials to furnish, fabricate and place the 8" thick precast concrete lagging at the proper locations indicated. Payment for this item will be made on a per SQUARE FOOT basis.

LINE ITEM 12 - Free Draining Backfill

This item will cover all excavation, furnishing & installation of backfill, compaction necessary and furnishing & installation filter fabric. Payment for this item will be made on a per TON basis.

LINE ITEM 13 – 6" Perforated Pipe

This item will cover furnishing & installation of the 6" perforated pipe embedded in the free draining backfill. Payment for this item will be made on a per LINEAR FOOT basis.

LINE ITEM 14 - Cable Guardrail

This item will cover all means, methods and materials to furnish, fabricate and place the cable guardrail at the proper locations indicated. Payment for this item will be made on a per LINEAR FOOT basis.

LINE ITEM 15 - Seed

This item will cover all means, methods and materials to furnish and place the seeding over all areas disturbed. Payment for this item will be made on a per ACRE basis.



State of West Virginia DRUG FREE WORKPLACE CONFORMANCE AFFIDAVIT West Virginia Code §21-1D-5

STATE OF WV
COUNTY OF Kanawha TO-WIT:
I, <u>Kelley D. Thaxton</u> , after being first duly sworn, depose and state as follows:
 I am an employee of <u>Thaxton Construction Co., Inc.</u>; and, (Company Name)
2. I do hereby attest that <u>Thaxton Construction Co., Inc.</u> (Company Name)
maintains a valid written drug free workplace policy and that such policy is in compliance with West Virginia Code §21-1D-5.
The above statements are sworn to under the penalty of perjury.
Thaxton Construction Co., Inc. (Company Name)
By: Malley O. Thartm
Title:
Date: 4/22/09
Taken, subscribed and sworn to before me this 22nd day of April, 2009
or sid Seel
By Commission expires the State of West Virginia Sharon S. Thexton Sharon S. Thexton Box 4 White Tail Lane Charleston, WV 25312 My commission expires July 4, 2011
My commission expires day (Notary Public)
ANTE MUST BE SUBMITTED WITH THE BID IN ORDER TO

COMPLY WITH WV CODE PROVISIONS. FAILURE TO INCLUDE THE AFFIDAVIT WITH THE BID SHALL RESULT IN DISQUALIFICATION OF THE BID. Rev March 2009

Drilled Shaft Wall Bid Form

> WVANG South Gate Road Slope Failure Addendum 2

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Unit Price	146,293.00	32,000.00	50,000.00	20,000.00	20,000.00	100.00	100.00	100.00	100.00	300.00	45.00	45.00	25.00	50.00	30.00	150.00	10.000.00	
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Quantity	1.00	0.25	1.00	1.00	1.00	136.00	272.00	40.00	481.57	360.00	680.00	200:00	2,070.00	150.00	230.00	280.00	0.50	F
Description	Mobilization and Demobilization	Clear & Grub	Excavation and Embankment	Erosion & Sediment Control	Construction Layout & Staking	Roadway Grading	Stone Base for Road	Soil Boring	30" Diameter Drilled Shafts, Above Bedrock	30" Diameter Drilled Shafts, Into Bedrock	Steel Piles, W18x106	Steel Piles, W21x111	Precast Concrete Lagging, 8" Thick	Free Draining Backfill	6" Perforated Pipe	Cable Guardrail	Seed	
Bid Item Number	1	2	8	4	5	9		8	6	10		12	13	14	15	16	11	

Contractor Name: Thaxton Construction Co., Inc.	Address: P.0. Box 13279	Charleston, WV 25360	Date: 4/22/09		the user O Thanks		Title: Vice President	
Contractor Name:	Address:	. 1	Date:	•	Signed:	Print Name:	Title:	



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State of West Virginia Department of Administration Purchasing Division 2019 Washington Street East Post Office Box 50130 Charleston, WV 25305-0130

Request for Quotation

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ADDRESS CORRESPONDENCE TO ATTENTION OF:

JOHN ABBOTT 304-558-2544

*B13150503 304-984-2299 THAXTON CONSTRUCTION CO INC SI PO BOX 13279 SISSONVILLE WV 25360

DIV ENGINEERING & FACILITIES CAMP DAWSON ARMY TRAINING SITE 240 ARMY ROAD

KINGWOOD, WV 26537-1077

304-329-4417

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# ADDENDUM NO. 1

# Pre-Bid Meeting Minutes

Contract DEFK9020

Engineer Skills Building, Camp Dawson, WV

The following constitutes Pre-bid Meeting Minutes for DEFK9020, Camp Dawson South Road Slip Repair.

A pre-bid meeting was conducted at the site for the referenced project at 1330 hrs 30 MAR 2009. During the pre-bid meeting, LTC Suver welcomed those present for expressing interest in the subject project. Key Owner representatives where introduced and the following agenda items were covered:

# 1. ADMINISTRATIVE:

- All present contractors signed in.
- b. The project is a federally funded, State administrated project.
- c. The user of the facility will be the West Virginia Army National Guard.
- d. The Administrator of the contract will be the Construction & Facilities Management Office, WVARNG.

# 2. Introduction:

- a. LTC Suver outlined key personnel associated with the project and address for the office. LTC Suver was introduced as the Administrative Contracting Officer, and Jeff Franklin as the Project Manager. Phone numbers and address were identified as follows:
- b.i. Address1703 Coonskin DriveCharleston, WV 25311
  - ii. LTC Bill Suver Administrative Contracting Officer Bill.suver@wv.ngb.army.mil (304) 561-6454

- iii. Jeff Franklin, Project Manager Email: jeff.franklin@wv.ngb.army.mil 791-4333 (o) (fax)
- c. John Abbott is the buyer for State Purchasing Division. All questions must be submitted in writing to Mr. Abbott, who will distribute for resolution. Direct discussion is **not** authorized with the Engineer, the Facilities Engineer, or the Project Manager. State Wage Rates applies for this project. Bid Opening is set for 4-15-2009. The contractors were reminded that they must submit the "Drug Free Compliance Affidavit" form with their bid; failure to submit this form will be grounds for automatic disqualification.
- 3. The Designer of Record, Chris Ramsey, P.E..
  - i. Address:
    AMEC Earth & Environmental
    3800 Ezell Road, Suite 100
    Nashville, TN 37211
    Ph: (615) 333-0630
- 4. LTC Suver discussed security, work hours, access to the site, and temporary facilities. The Contractor will have access to the site from 0700 to 1800 hrs, Monday to Friday. If work hours must be modified, these will be approved on a case-by-case via the Project Manager (CPT Franklin). The contractor is required to provide to the Superintendent, a listing of personnel, which will be gaining access to the site. Superintendence: In accordance with Contract documents, the Contractor must maintain full-time, active superintendent on the job. The contractor will provide a port-a-john for all personnel involved with Construction.
- 5. Contract Duration 60 days to complete the project after the NTP.
- 6. All construction activities will be in conjunction with OHSA, Safety and Health Requirements Manual (US Army Corps of Engineers). As noted, the Army safety and health standards mirror OHSA. All construction activities on site will be a hardhat area and marked as such. Cleanup is required daily by each perspective sub and General Contractor: No open dumps of construction materials and no burning on site.
- 7. LTC Suver discussed the submittal process and encouraged the contractors to stay with the manufacturers outlined in the specifications and any proposed substitution must be submitted prior to bidding.
- 8. The meeting was opened for Questions by the Contractors. See attached addendum items.
- 9. LTC Suver closed the meeting and thanked the Contractors for their interest in the project. LTC Suver outlined that the meeting notes will be published through state purchasing, along with clarifications to contract documents. Any questions will be addressed through an Addendum released after the question period has closed. It was also stressed that any further questions between now and bid award must be directed to John Abbott at State Purchasing.

# Clarifications from the 3/30/09 Pre-Bid Meeting

- 1. Specified concrete to be used on this project should be a Class B modified, 4,000 psi in 28 days.
- 2. Existing concrete rubble onsite and drilling spoils may be disposed of on WVARNG property at designated area, see attached aerial. Erosion and sediment control measures must be followed in this area.
- 3. All rubbish, clearing & grubbing material, dunnage, etc. must be disposed of at an off site location at the contractor's expense.
- 4. The subsurface exploration did not extend far enough south, so an interpolated bedrock surface was used. Bid items have been revised to establish the best bidding situation for all.
- 5. On site materials may be re-used for grading and backfill purposes, as long as compaction criteria can be met. Any clean excess soil or gravel may be dispose of on Camp Dawson as directed by the owner, maximum travel distance 2 miles, one way.

# Contractor Questions

- Q: The location of the new wall is not referenced off the centerline of the road in the cross sections or typical section. On the location plan sheet there is a 17.8' distance shown from a dashed line, is this from the east ditch line to back of pile?
- A: The distance called out on sheet DP-3 is 17'-8", and is measured from the east ditch line to the back of the precast concrete lagging.
- Q: Where and how does the 6" drain outlet?
- A: The 6" drain starts on the higher elevation part of the wall, STA 13+85, and provides positive drainage down to where it 'daylights' on the lower elevation part of the wall, at STA 11+50. This is indicated on sheet DP-6, Typical Section.
- Q: The RFQ states that the contract is to be performed within 60 calendar days after the notice to proceed is received. Will the start of the 60 day time period be modified to begin after obtaining the necessary permits and material as discussed at the pre-bid meeting?
- A: The contract period will be based on when the material will be delivered to the site and the contractor can begin the work.

Drilled Shaft Wall Bid Form

WVANG South Gate Road Slope Failure Addendum 1

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Bid Item	Description	Quantity	Gnit	Unit Price	Total Price
Number		,			***************************************
-	Mobilization and Demobilization	1.00	ട്ട		٠
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5	Roadway Grading	136.00	ζ		ج
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6	30" Diameter Drilled Shafts, Into Bedrock	360.00	느		•
10	Steel Piles. W18x106	680.00	LЫ		\$
-	Steel Piles, W21x111	200.00	LET		ج
12	Precast Concrete Lagging, 8" Thick	2,070.00	SFT		۱ \$
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15	Cable Guardrail	280.00	LLI		٠
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Contractor Name: Address:	Date:	Signed: Print Name: Title:

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Email Address: 304 964 2299		FAX 307 984 23.34
Company: Tormen Co-struction	Do, Box 460 Ranbousonle WU 25504	PHONE 304-525-9485 TOLL FREE
Email Address: NE76 Boize @ aol-com		FAX 304-525-9181
Company: JL Pretzel Contracting	9 Belging Est. Bruceton Mills We 26525	PHONE 304-379-7789 TOLL FREE
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Rep: Todd E BALCAR	BELMON OH 43718	FREE
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Email Address: hawkeyr @ Mountain - MET		FAX 304 (81 6266
COMPANY. CHARLES E. ROLYARD & SON INC	125 EAST HOURST	PHONE 304 - 329 - 1330 TOLL
Rep. But BowyARD	KANDINION WW 2653	FREE TASA TASA - 1571
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Email Address: SCalucat GR & @ Mol. Com		FAX 304-594-3992
Company: ORANGE CONSTRUCTION LORP.	NORGANTOWN WV 26508	PHONE 304-291-676く TOLL FREE
		FAX 304-291-6975
Company: Drawing Cowstructions Cor	11 11	PHONE ZE/ 6765 TOLL FREE
		FAX 251 6975
Company: LAVAITG BACAUQTING	302 Dents RD MORGANTOWN WU.	PHONE 304-296-7531 TOLL FREE
Email Address: FREEMAN @ LAWAITS COW	90506	FAX 304-292-4606
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State of West Virginia Department of Administration Purchasing Division 2019 Washington Street East Post Office Box 50130 Charleston, WV 25305-0130

# Request for Quotation

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JOHN ABBOTT 304-558-2544

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*B13150503 304-984-2299 THAXTON CONSTRUCTION CO INC SI PO BOX 13279

SISSONVILLE WV 25360

DIV ENGINEERING & FACILITIES CAMP DAWSON ARMY TRAINING SITE 240 ARMY ROAD

KINGWOOD, WV 26537-1077

304-329-4417

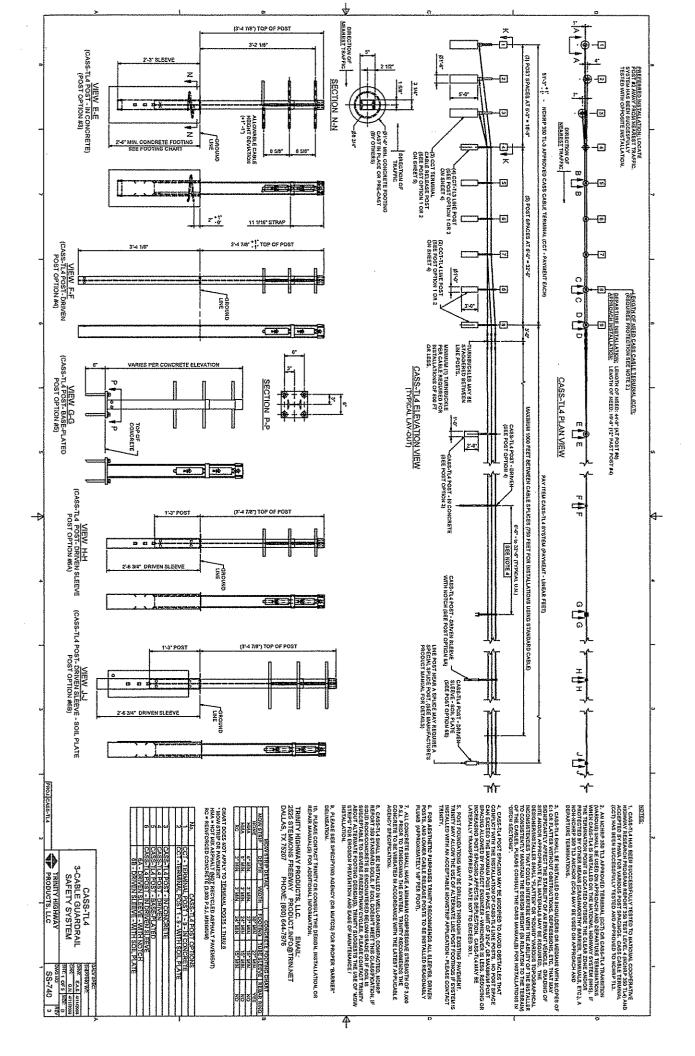
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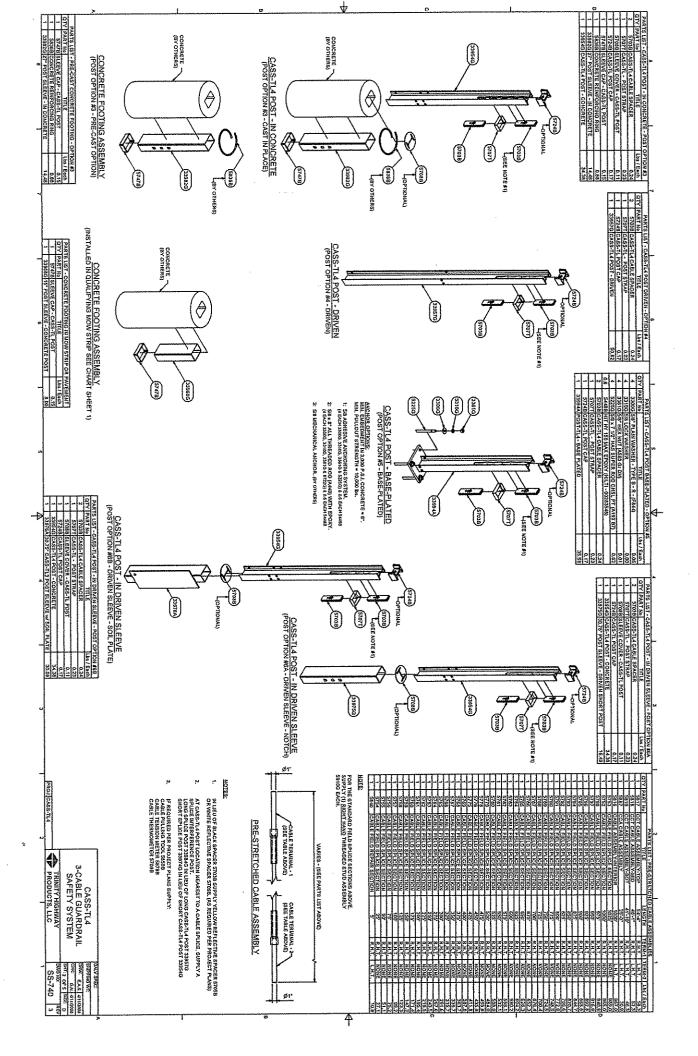
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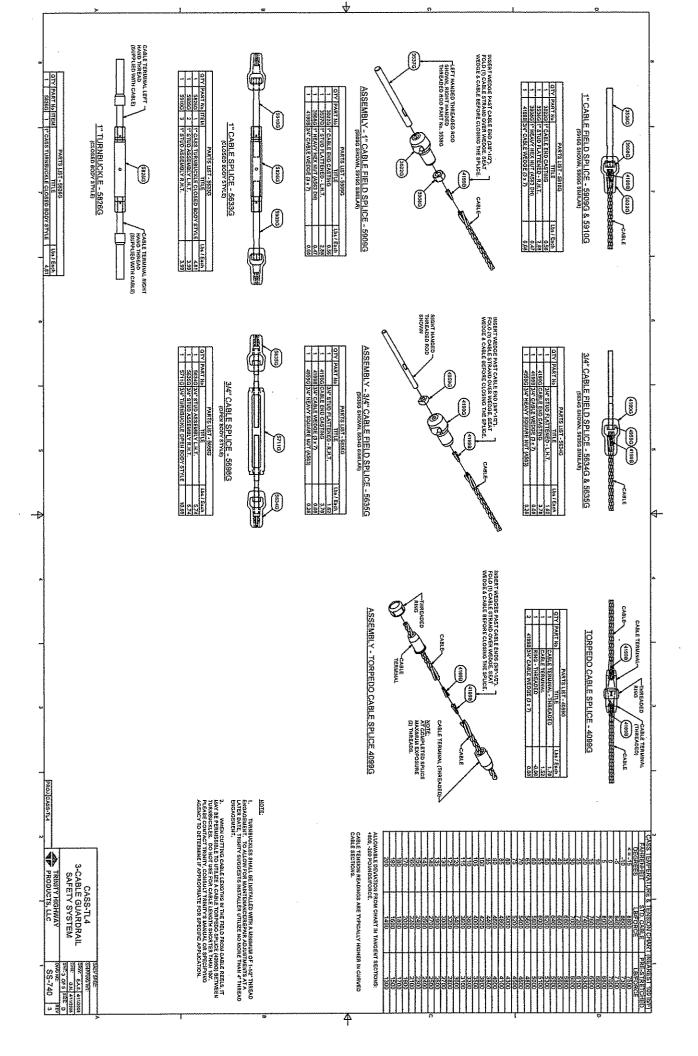
- Q: Within addendum #1 Clarification from the pre-bid meetings, #2, it is stated that the onsite rubble can be hauled to an area that is shown on an attached aerial. The attached aerial cannot be found.
- A: Please refer to the narrative describing this area included in Addendum #1 and repeated herein.
- Q: Should we not have a detail of just what the engineer is wanting for the cable guard rail system?
- A: Please refer to the attached five sheet set of drawing number SS-740 of the CASS TL-4 Cable Guardrail system produced by Trinity Highway Products. Provide this system or equivalent. Assume installation will require post option #1.
- Q: In the pre-bid meeting we had a discussion of allowing the successful bidder to perform an additional core hole at the upper end of the wall prior to ordering material at the expense of the owner. I see no mention of this in the Addendum #1.
- A: A Line Item was added to the bid form for a 40' deep soil boring in Addendum #1.

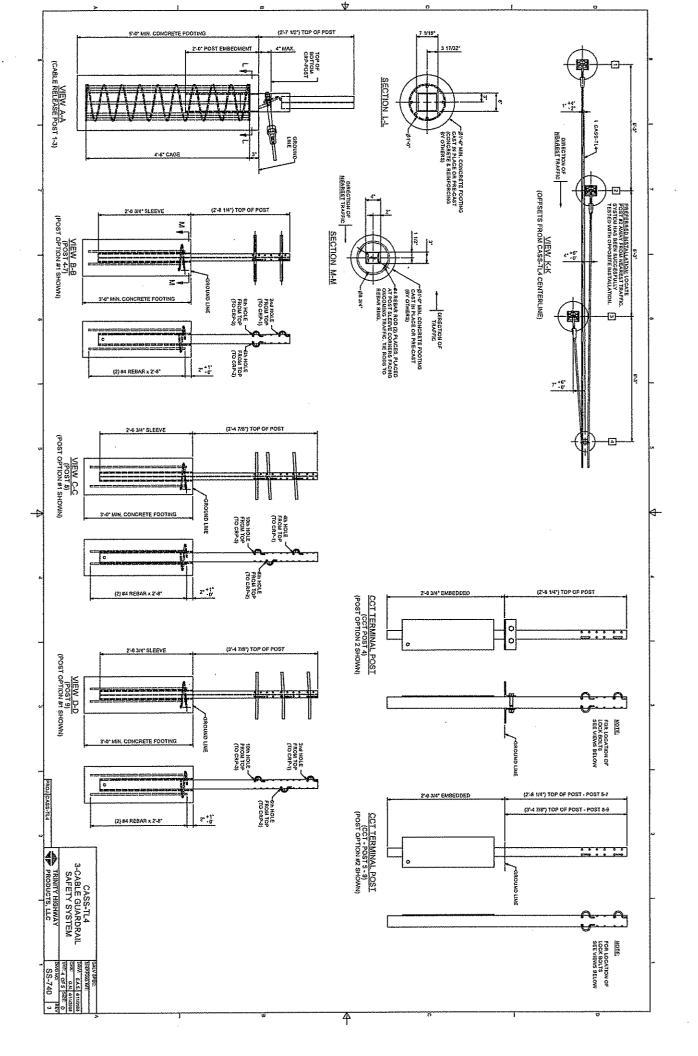
# Bid Form Update

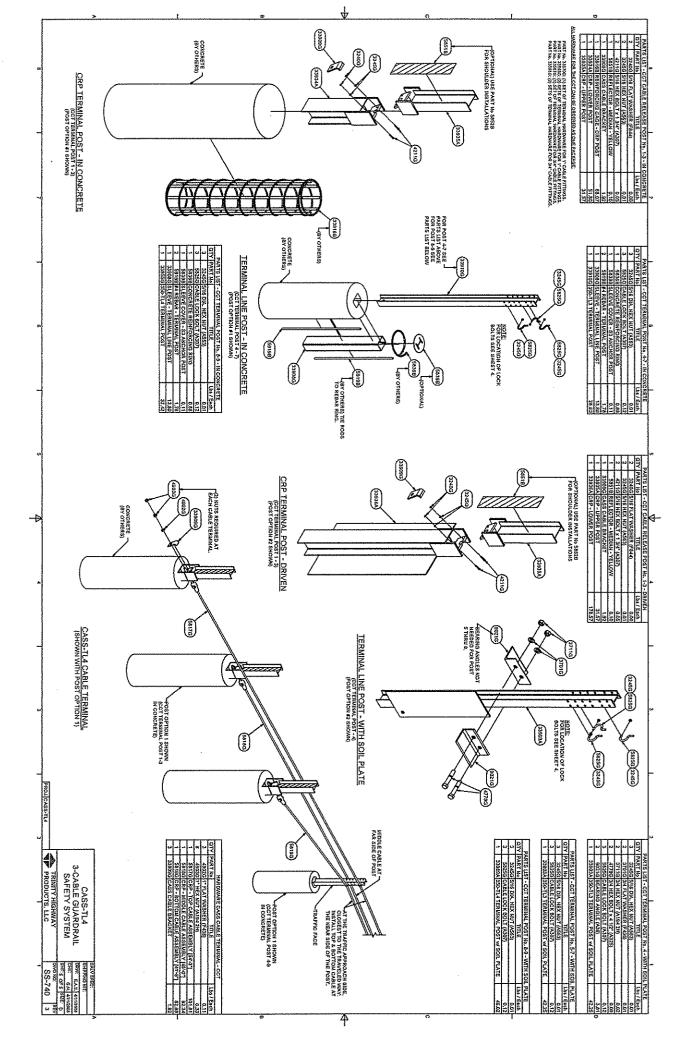
The bid form has been updated to include a Lump Sum line item for Construction Layout & Staking. The Owner will provide control points and benchmarks, from which the Contractor will provide all necessary surveying required to complete the Work.











	on of Engineering &
Agency	Facilities
REQ.P.O#	

# **BID BOND**

	KNOW	ALL MEN BY TH	YESE PRESENTS, Th	et we, the undersigned,	Thaxton Construct	ion Co., Inc. of Sissonville
		P.O. Box 13279		ston, WV 26360	_, as Principal, and _	International Fidelity Insurance
Compa	any of	Newark	. New Jersey	, a corporation o	rganized and existing	under the laws of the State of
					_, as Surety, are held	d and firmly bound unto the State
of West	. Virginia	, as Obligee, in th	ne penal sum of <u>five</u>	percent of bid	(\$5%	) for the payment of which,
well an	d truly to	be made, we joir	itly and severally blind	ourselves, our heirs, adr	ninistrators, executors	s, successors and assigns.
			·			e Purchasing Section of the
			, ,		•	nter into a contract in writing for
				Contract to repair the roa	d silp at the vvest vii	ginia National Guard,
Camp	Dawson	, Kingwood, Wes	i virginia			
	WOM	THEREFORE,				
		said bid shall be i				
hannta	(b) If	said bid shall be a	sccepted and the Prince	sipal shall enter into a co	ntract in accordance v	with the bid or proposal attached all other respects perform the
nereto -	ano snei ient crea	if turnish any bine ited by the accept	ance of said bid, then	this obligation shall be n	ull and void, otherwise	e this obligation shall remain in full
force at	nd effect	. It is expressly u	nderstood and agreed	that the liability of the S	urety for any and all c	laims hereunder shall, in no event,
exceed	the pen	al amount of this	obligation as herein at	aled.		
	ای مدالا	urely for the value	o receiumd hereby stir	ulates and acrees that II	ne obligations of said	Surety and its bond shall be in no
	paired o	r affected by any	extension of the time v	vithin which the Obligee	may accept such bid,	and said Surety does hereby
waiver	notice of	any such extensi	on.			
	iN Wi	TNESS WHEREO	F. Principal and Suret	y have hereunto set their	hands and seals, an	d such of them as are corporations
have ca	aused th	eir corporate seal	s to be affixed hereun	o and these presents to	be signed by their pro	per officers, this
22nd_	day of	April	, 20 09 ,			
					Thayton Construct	ion Co., Inc. of Sissonville
Princip	el Corpo	rate Seal				Name of Principal)
					4/	Mark
					Ву	Must be President or
						Vice President)
						President
						(Title)
Surety	Corpora	te Seal				y Insurance Company
•						Name of Surety)
					MA.	AAA
					Br. AllX	$(\mathcal{N})$
					U	Atterney-in-Fact

IMPORTANT - Surety executing bonds must be licensed in West Virginia to transact surely insurance. Corporate seals must be affixed, and a power of attorney must be attached.

# POWER OF ATTORNEY

# INTERNATIONAL FIDELITY INSURANCE COMPANY

HOME OFFICE: ONE NEWARK CENTER, 20TH FLOOR NEWARK, NEW JERSEY 07102-5207

# FOR BID BOND/RIDER/CONSENTS/AFFIDAVITS

KNOW ALL MEN BY THESE PRESENTS: That INTERNATIONAL FIDELITY INSURANCE COMPANY, a corporation organized and existing laws of the State of New Jersey, and having its principal office in the City of Newark, New Jersey, does hereby constitute and appoint

C. DAVID THOMAS, RICHARD L. HIGGINBOTHAM, ROSEANN B. DYE-SMALLEY, BUNNIE MARIE PERRINE, JEFFERY O'DELL. ROBIN HUBBARD-SHERROD

Charleston, WV.

its true and lawful attorney(s)-in-fact to execute, seal and deliver for and on its behalf as surety, any and all bonds and undertakings, contracts of indemnity and other writings obligatory in the nature thereof, which are or may be allowed, required or permitted by law, stature, rule, regulation, contract or otherwise, and the execution of such instrument(s) in pursuance of these presents, shall be as binding upon the said INTERNATIONAL FIDELITY INSURANCE COMPANY, as fully and amply, to all intents and purposes, as if the same had been duly executed and acknowledged by its regularly elected officers at its principal office.

This Power of Attorney is executed, and may be revoked, pursuant to and by authority of Article 3-Section 3, of the By-Laws adopted by the Board of Directors of INTERNATIONAL FIDELITY INSURANCE COMPANY at a meeting called and held on the 7th day of February, 1974.

The President or any Vice President, Bxecutive Vice President, Secretary or Assistant Secretary, shall have power and authority

- (1) To appoint Attorneys-in-fact, and to authorize them to execute on behalf of the Company, and attach the Seal of the Company thereto, bonds and undertakings, contracts of indemnity and other writings obligatory in the nature thereof and,
- (2) To remove, at any time, any such attorney-in-fact and revoke the authority given.

Further, this Power of Attorney is signed and sealed by facsimile pursuant to resolution of the Board of Directors of said Company adopted at a meeting duly called and held on the 29th day of April, 1982 of which the following is a true excerpt:

Now therefore the signatures of such officers and the seal of the Company may be affixed to any such power of attorney or any certificate relating thereto by facsimile, and any such power of attorney or certificate bearing such facsimile signatures or facsimile seal shall be valid and binding upon the Company and any such power so executed and certified by facsimile signatures and facsimile seal shall be valid and binding upon the Company in the future with respect to any bond or undertaking to which it is attached.

SEAL THOUSE SEAL T

IN TESTIMONY WHEREOF, INTERNATIONAL FIDELITY INSURANCE COMPANY has caused this instrument to be signed and its corporate seal to be affixed by its authorized officer, this 16th day of October, A.D. 2007.

INTERNATIONAL FIDELITY INSURANCE COMPANY

STATE OF NEW JERSEY County of Essex

Secretary

On this 16th day of October 2007, before me came the individual who executed the preceding instrument, to me personally known, and, being by me duly sworn, said the he is the therein described and authorized officer of the INTERNATIONAL FIDELITY INSURANCE COMPANY; that the seal affixed to said instrument is the Corporate Seal of said Company; that the said Corporate Seal and his signature were duly affixed by order of the Board of Directors of said Company.



IN TESTIMONY WHEREOF, I have hereunto set my hand affixed my Official Seal, at the City of Newark, New Jersey the day and year first above written.

A NOTARY PUBLIC OF NEW JERSEY My Commission Expires Nov. 21, 2010

aria H. Granco

CERTIFICATION

I, the undersigned officer of INTERNATIONAL FIDELITY INSURANCE COMPANY do hereby certify that I have compared the foregoing copy of the Power of Attorney and affidavit, and the copy of the Section of the By-Laws of said Company as set forth in said Power of Attorney, with the ORIGINALS ON IN THE HOME OFFICE OF SAID COMPANY, and that the same are correct transcripts thereof, and of the whole of the said originals, and that the said Power of Attorney has not been revoked and is now in full force and effect

IN TESTIMONY WHEREOF, I have hereunto set my hand this 22nd day of April, 2009

Assistant Secretary