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State of West Virginia
Department of Administration
Purchasing Division
2019 Washington Street East Post Office Box 50130 Charleston, WV 25305-0130

Request for Quotation

DEFK9020

ADDRESS CORRESEO	NUENCE TO ATTENTION OF
JOHN ABBOTT	
304-558-2544	

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DIV ENGINEERING & FACILITIES CAMP DAWSON ARMY TRAINING SITE 240 ARMY ROAD

KINGWOOD, WV 26537-1077

304-329-4417

RFQ COPY TYPE NAME/ADDRESS HERE Laurita Excavating, Inc.

302 Dents Run Road Morgantown, WV 26501

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GENERAL TERMS & CONDITIONS REQUEST FOR QUOTATION (RFQ) AND REQUEST FOR PROPOSAL (RFP)

- 1. Awards will be made in the best interest of the State of West Virginia.
- 2. The State may accept or reject in part, or in whole, any bid.
- 3. All quotations are governed by the West Virginia Code and the Legislative Rules of the Purchasing Division.
- 4. Prior to any award, the apparent successful vendor must be properly registered with the Purchasing Division and have paid the required \$125 fee.
- 5. All services performed or goods delivered under State Purchase Order/Contracts are to be continued for the term of the Purchase Order/Contracts, contingent upon funds being appropriated by the Legislature or otherwise being made available. In the event funds are not appropriated or otherwise available for these services or goods, this Purchase Order/Contract becomes void and of no effect after June 30.
- 6. Payment may only be made after the delivery and acceptance of goods or services.
- 7. Interest may be paid for late payment in accordance with the West Virginia Code.
- 8. Vendor preference will be granted upon written request in accordance with the West Virginia Code.
- 9. The State of West Virginia is exempt from federal and state taxes and will not pay or reimburse such taxes.
- 10. The Director of Purchasing may cancel any Purchase Order/Contract upon 30 days written notice to the seller.
- 11. The laws of the State of West Virginia and the *Legislative Rules* of the Purchasing Division shall govern all rights and duties under the Contract, including without limitation the validity of this Purchase Order/Contract.
- 12. Any reference to automatic renewal is hereby deleted. The Contract may be renewed only upon mutual written agreement of the parties.
- 13. BANKRUPTCY: In the event the vendor/contractor files for bankruptcy protection, this Contract may be deemed null and void, and terminated without further order.
- 14. HIPAA BUSINESS ASSOCIATE ADDENDUM: The West Virginia State Government HIPAA Business Associate Addendum (BAA), approved by the Attorney General, and available online at the Purchasing Division's web site (http://www.state.wv.us/admin/purchase/vrc/hipaa.htm) is hereby made part of the agreement. Provided that, the Agency meets the definition of a Cover Entity (45 CFR §160.103) and will be disclosing Protected Health Information (45 CFR §160.103) to the vendor.
- 15. WEST VIRGINIA ALCOHOL & DRUG-FREE WORKPLACE ACT: If this Contract constitutes a public improvement construction contract as set forth in Article 1D, Chapter 21 of the West Virginia Code ("The West Virginia Alcohol and Drug-Free Workplace Act"), then the following language shall hereby become part of this Contract: "The contractor and its subcontractors shall implement and maintain a written drug-free workplace policy in compliance with the West Virginia Alcohol and Drug-Free Workplace Act, as set forth in Article 1D, Chapter 21 of the West Virginia Code. The contractor and its subcontractors shall provide a sworn statement in writing, under the penalties of perjury, that they maintain a valid drug-free work place policy in compliance with the West Virginia and Drug-Free Workplace Act. It is understood and agreed that this Contract shall be cancelled by the awarding authority if the Contractor: 1) Fails to implement its drug-free workplace policy; 2) Fails to provide information regarding implementation of the contractor's drug-free workplace policy at the request of the public authority; or 3) Provides to the public authority false information regarding the contractor's drug-free workplace policy."

INSTRUCTIONS TO BIDDERS

- 1. Use the quotation forms provided by the Purchasing Division.
- 2. SPECIFICATIONS: Items offered must be in compliance with the specifications. Any deviation from the specifications must be clearly indicated by the bidder. Alternates offered by the bidder as EQUAL to the specifications must be clearly defined. A bidder offering an alternate should attach complete specifications and literature to the bid. The Purchasing Division may waive minor deviations to specifications.
- 3. Complete all sections of the quotation form.
- 4. Unit prices shall prevail in case of discrepancy.
- 5. All quotations are considered F.O.B. destination unless alternate shipping terms are clearly identified in the quotation.
- 6. BID SUBMISSION: All quotations must be delivered by the bidder to the office listed below prior to the date and time of the bid opening. Failure of the bidder to deliver the quotations on time will result in bid disqualifications: Department of Administration, Purchasing Division, 2019 Washington Street East, P.O. Box 50130, Charleston, WV 25305-0130



State of West Virginia
Department of Administration
Purchasing Division
2019 Washington Street East Post Office Box 50130 Charleston, WV 25305-0130

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JOHN ABBOTT 304-558-2544

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OHN ABBOTT	
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JOHN ABBOTT 304-558-2544

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> Laurita Excavating, Inc. 302 Dents Run Road Morgantown, WV 26501

DIV ENGINEERING & FACILITIES CAMP DAWSON ARMY TRAINING SITE 240 ARMY ROAD

KINGWOOD, WV 26537-1077

DATE PRINT	ED	TER	MS OF SALE		SHIP VIA		Fi	j.B	FREIGHTTERMS
03/12/	2009								
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Projec	ct Name:	WVARNG	South Gate
Road	<del>-</del>		
Projec	t No. <u>7-7</u>	728-0000	
Bw	CJR		

By: <u>CJR</u> Chk'd By: <u>DWD</u>

# WVARNG South Gate Road Slope Repair Design Brief

Dated: 2/25/09

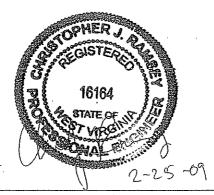
Prepared For:

WVARNG



Prepared By:

amec



File: South Gate Road Design

Brief.xmcd

Page 1 of 11



Project Name:	WVARNG South Gate						
Road_							
Project No. <u>7-7728-0000</u>							
_							
By: CJR							
Chk'd By:	OWD						

**Design Discussion** 

The South Gate Access Road has suffered from intermittent erosion and minor landslides, which have led to a state of disrepair and limited use. AMEC prepared three concepts for stabilization of approximately 230 linear feet of the road. AMEC recommended, and WVARNG concurred that the best method of repair was a drilled shaft wall embedded into bedrock with precast concrete lagging spanning between the shafts near the surface. Multiple exploratory borings and surface reconnaissance were used to estimate the limits of the proposed drilled shaft retaining wall, the loads that would be induced on the system as it supports the restored road grade and adjacent hillside, and the soil & rock properties within the underlying subsurface profile.

Lab tests indicate an average unit weight for the overburden soils is approximately 115 pounds per cubic foot (pcf). Considering the amount of clay content in soil matrix, an appropriate friction angle would be 25 degrees. Field and lab inspection of the rock core samples indicate a hard shale, that would most likely not be easily removed with normal augering tools. As such, we assigned a unit weight of 140 pcf, a friction angle of 40 degrees and a shear strength of 10,000 pounds per square foot to the rock

Drilled shaft retaining walls are common in roadsides next to river valley applications, similar to this. The methodology developed to design this system is derived from the following sources:

- AASHTO Standard Specifications for Bridges, 17th Edition
- FHWA Publication Geotechnical Engineering Circular No. 4 (1999), and
- "Slide Control by Drilled Pier Walls", M. Nethero, ASCE National Convention; Las Vegas, NV; 1982.

File: South Gate Road Design Brief.xmcd

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Project Name: <u>WVARNG South Gate</u> Road

Project No. _7-7728-0000

By: ___CJR Chk'd By: DWD

#### Drilled Shaft Retaining Wall - Design Section 11+00:

#### Soil Parameters:

Unit weight

 $\gamma_{\text{soil}} := 115 \cdot \text{pcf}$ 

Phi angle  $\phi_{soil} := 25 \cdot \text{deg}$   $k_{a_soil} := \tan \left( 45 \cdot \text{deg} - \frac{\phi_{soil}}{2} \right)^2$   $k_{a_soil} = 0.41$ FS := 1.5  $k_{p_soil} := \frac{1}{k_{a_soil}}$   $k_{p_soil} = 2.46$ 

Factor of Safety on Passive

FS := 1.5

#### **Rock Parameters:**

Unit weight

 $\gamma_{\text{rock}} := 140 \cdot \text{pcf}$ 

Phi angle

 $\varphi_{rock} := 40 \cdot deg$ 

 $k_{a_rock} := tan \left( 45 \cdot deg - \frac{\phi_{rock}}{2} \right)^2$ 

Factor of Safety on Passive

FS := 1.5

 $k_{p_rock} := \frac{1}{k_{p_rock}} \qquad k_{p_rock} = 4.60$ 

Rock Shear Strength

 $C_{rock} := 10 ksf$ 

#### Wall Characteristics:

Wall height

 $H := 27 \cdot ft$ 

Depth of Overburden h := 18ft

Pile Width

 $b_f := 2.5 \cdot ft$  Pile spacing  $s := 10 \cdot ft$ 

Horizontal Surcharge Pressure applied

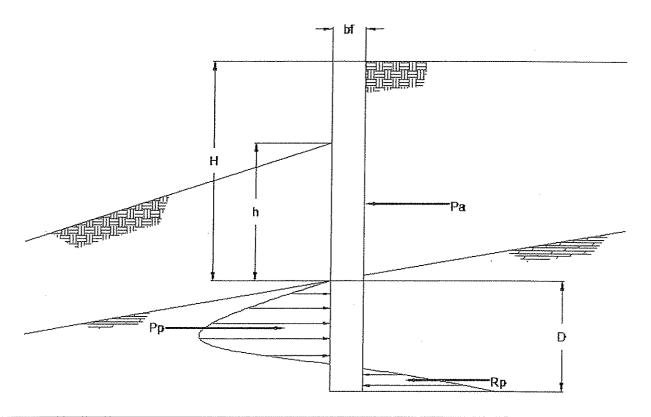
q := 100psf

 $b_e := if(3 \cdot b_f < s, 3 \cdot b_f, s)$ 

 $b_e = 7.5 \, ft$ 

Depth of Rock Socket

D := 12ft



File: South Gate Road Design

Brief.xmcd

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Project Name: <u>WVARNG South Gate</u>
Road
Project No. <u>7-7728-0000</u>

By: <u>CJR</u> Chk'd By: <u>DWD</u>

$$P_a := k_{a_soil} \cdot \gamma_{soil} \cdot \frac{(H)^2}{2} = 17.01 \cdot \frac{kips}{ft}$$

SUM MOMENTS ABOUT "P" AND SOLVE FOR "Rp"

$$R_p := \left[ \left( P_a \cdot s \right) \cdot \left( \frac{H}{3} + \frac{D}{3} \right) - \gamma_{soil} \cdot \left[ \frac{k_{p_soil} \cdot b_e}{FS} \cdot \left[ \frac{\left(h\right)^2}{2} \right] \right] \cdot \left( \frac{h}{3} + \frac{D}{3} \right) + q \cdot H \cdot s \cdot \left( \left( \frac{H}{2} + \frac{D}{3} \right) \right) \right] \cdot \frac{9}{D \cdot 5} = 58.35 \cdot \text{kips}$$

SUM OF THE HORIZONTAL FORCES = 0, DETERMINE "Pp"

$$P_p := P_{a} \cdot s - \left[ \gamma_{soil} \cdot \frac{k_{p_soil} \cdot b_e}{FS} \cdot \left( \frac{h^2}{2} \right) \right] + q \cdot H \cdot s + R_p = 25.96 \cdot kips$$

$$P_{p_avg} := \frac{P_p}{\frac{2 \cdot D \cdot b_f}{3}} = 1.3 \cdot ksf$$

$$P_{p_{max}} := 1.5 \cdot P_{p_{avg}} = 1.95 \cdot ksf$$

DUE TO PARABOLIC DISTRIBUTION

$$R_{p_avg} := \frac{R_p}{D \cdot \frac{b_f}{3}} = 5.83 \cdot ksf$$

$$R_{p_max} := 2 \cdot R_{p_avg} = 11.67 \cdot ksf$$

DUE TO TRIANGULAR DISTRIBUTION



Project Name: WVARNG South Gate
Road
Project No. 7-7728-0000

By: <u>CJR</u> Chk'd By: <u>DWD</u>

ULTIMATE PASSIVE PRESSURE @ Pp or D/3

$$D3_{pass} := \gamma_{soil} \cdot H + 2 \cdot C_{rock} + \gamma_{rock} \cdot \frac{D}{3} = 23.66 \cdot ksf$$

$$FS_{D3_pass} := \frac{D3_{pass}}{P_{p max}} = 12.15$$

OK!

ULTIMATE PASSIVE PRESSURE @ Rp or 8D/9

$$D_{8_9pass} := \gamma_{soil} \cdot H + 2 \cdot C_{rock} + \gamma_{rock} \cdot \frac{8D}{9} = 24.6 \cdot ksf$$

$$FS_{D8_{9pass}} := \frac{D_{8_{9pass}}}{R_{p_{avg}}} = 4.22$$

2.0

2.0

OK!

ULTIMATE PASSIVE PRESSURE @ D

$$D_{pass} := \gamma_{soil} \cdot H + 2 \cdot C_{rock} + \gamma_{rock} \cdot D = 24.79 \cdot ksf$$

$$FS_{Dpass} := \frac{D_{pass}}{R_{p max}} = 2.12$$

2.0

OK!

FIND POINT OF ZERO SHEAR

$$V_o := \frac{\left[P_a \cdot s - \left[\gamma_{soil} \cdot \frac{k_{p_soil} \cdot b_e}{FS} \cdot \left(\frac{h^2}{2}\right)\right] + q \cdot H \cdot s\right]}{b_f \cdot P_{p_avg}} = -9.98 \text{ ft}$$

ABOVE TOP OF ROCK

MAXIMUM MOMENT

$$M_{max} := \left(\frac{H}{3} + V_o\right) \cdot P_a \cdot s - \left(\frac{h}{3} + V_o\right) \cdot \left[\gamma_{soil} \cdot \frac{k_{p_soil} \cdot b_e}{FS} \cdot \left(\frac{h^2}{2}\right)\right] + \left(\frac{H}{2} + V_o\right) \cdot q \cdot H \cdot s - P_{p_avg} \cdot b_f \cdot \frac{{V_o}^2}{2} = 680.16 \cdot ft \cdot kips$$

$$F_v := 50$$
ksi

$$F_a := .66F_v = 33 \cdot ksi$$

$$S_{x_rqd} := \frac{M_{max}}{F_a} = 247.33 \cdot in^3$$

USE W21x111 GR50 PILE



Project Name: WVARNG South Gate

Road

Project No. <u>7-7728-0000</u>

Chk'd By: _

DWD

### Drilled Shaft Retaining Wall - Design Sections 12+00 & 13+00:

#### Soil Parameters:

Unit weight

 $\gamma_{soil} \coloneqq 115 {\cdot} pcf$ 

Phi angle  $\phi_{soil} := 25 \cdot \deg$ 

 $k_{a_soil} := tan \left( 45 \cdot deg - \frac{\Phi_{soil}}{2} \right)^2$   $k_{a_soil} = 0.41$   $k_{p_soil} := \frac{1}{k_{a_soil}}$   $k_{p_soil} = 2.46$ 

Factor of Safety on Passive

FS := 1.5

#### **Rock Parameters:**

Unit weight

 $\gamma_{rock} \coloneqq 140 \cdot pcf$  . Phi angle  $\varphi_{rock} \coloneqq 40 \cdot deg$ 

 $k_{a_rock} := \tan\left(45 \cdot \deg - \frac{\Phi_{rock}}{2}\right)^2$ 

Factor of Safety on Passive

FS := 1.5

 $k_{p_rock} := \frac{1}{k_{a_rock}} \qquad k_{p_rock} = 4.60$ 

**Rock Shear Strength** 

 $C_{rock} := 10ksf$ 

#### Wall Characteristics:

Wall height

H := 20-ft

Depth of Overburden h := 11ft

Pile Width

 $b_f := 2.5 \cdot ft$  Pile spacing

 $s := 10 \cdot ft$ 

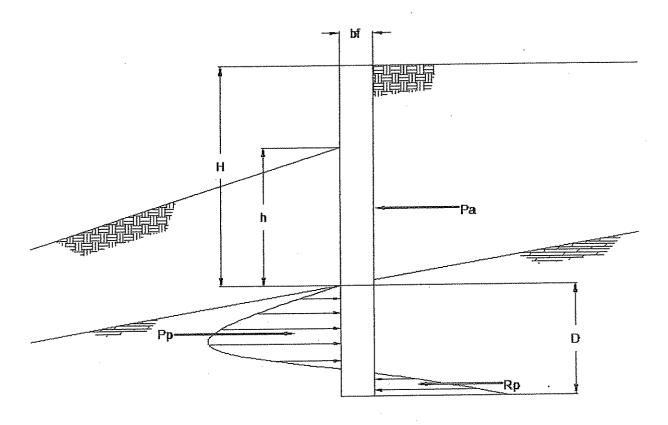
Horizontal Surcharge Pressure applied

q := 100psf

 $b_e := if(3 \cdot b_f < s, 3 \cdot b_f, s)$   $b_e = 7.5 ft$ 

Depth of Rock Socket

D := 15ft



File: South Gate Road Design Brief.xmcd

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Project Name: WVARNG South Gate
Road

Project No. _7-7728-0000

By: ___CJR Chk'd By: ___DWD____

$$P_a := k_{a_soil} \cdot \gamma_{soil} \cdot \frac{(H)^2}{2} = 9.33 \cdot \frac{kips}{ft}$$

SUM MOMENTS ABOUT "P" AND SOLVE FOR "Rp"

$$R_p := \left[ \left( P_a \cdot s \right) \cdot \left( \frac{H}{3} + \frac{D}{3} \right) - \gamma_{soil} \cdot b_e \cdot \left[ \frac{k_{p_soil}}{FS} \cdot \left[ \frac{\left( h \right)^2}{2} \right] \right] \cdot \left( \frac{h}{3} + \frac{D}{3} \right) + q \cdot H \cdot s \cdot \left( \left( \frac{H}{2} + \frac{D}{3} \right) \right) \right] \cdot \frac{9}{D \cdot 5} = 77.54 \cdot \text{kips}$$

SUM OF THE HORIZONTAL FORCES = 0, DETERMINE "Pp"

$$P_p := P_a \cdot s - \left[ \gamma_{soil} \cdot b_e \cdot \frac{k_{p_soil}}{FS} \cdot \left( \frac{h^2}{2} \right) \right] + q \cdot H \cdot s + R_p = 105.18 \cdot kips$$

$$P_{p_avg} := \frac{P_p}{\frac{2 \cdot D \cdot b_f}{3}} = 4.21 \cdot ksf$$

$$P_{p_max} := 1.5 \cdot P_{p_avg} = 6.31 \cdot ksf$$

$$R_{p_avg} := \frac{R_p}{D \cdot \frac{b_f}{3}} = 6.2 \cdot ksf$$

$$R_{p_max} := 2 \cdot R_{p_avg} = 12.41 \cdot ksf$$



Project Name: WVARNG South Gate

Road

Project No. 7-7728-0000

By: <u>CJR</u>

Chk'd By: <u>DWD</u>

ULTIMATE PASSIVE PRESSURE @ Pp or D/3

$$D3_{pass} := \gamma_{soil} \cdot H + 2 \cdot C_{rock} + \gamma_{rock} \cdot \frac{D}{3} = 23 \cdot ksf$$

$$FS_{D3_pass} := \frac{D3_{pass}}{P_{p_max}} = 3.64$$

2.0

2.0

OK!

ULTIMATE PASSIVE PRESSURE @ Rp or 8D/9

$$D_{8_9pass} := \gamma_{soil} \cdot H + 2 \cdot C_{rock} + \gamma_{rock} \cdot \frac{8D}{9} = 24.17 \cdot ksf$$

$$FS_{D8_9pass} := \frac{D_{8_9pass}}{R_{p_avg}} = 3.9$$
 >

OK!

ULTIMATE PASSIVE PRESSURE @ D

$$D_{pass} := \gamma_{soil} \cdot H + 2 \cdot C_{rock} + \gamma_{rock} \cdot D = 24.4 \cdot ksf$$

$$FS_{Dpass} := \frac{D_{pass}}{R_{p,max}} = 1.97$$
 > 2.0 OK!

FIND POINT OF ZERO SHEAR

$$V_{o} := \frac{\left[P_{a} \cdot s - \left[\gamma_{soil} \cdot b_{e} \cdot \frac{k_{p_soil}}{FS} \cdot \left(\frac{h^{2}}{2}\right)\right] + q \cdot H \cdot s\right]}{b_{f} \cdot P_{p_avg}} = 2.63 \, \text{ft} \qquad \text{ABOVE TOP OF ROCK}$$

MAXIMUM MOMENT

$$M_{max} := \left(\frac{H}{3} + V_o\right) \cdot P_a \cdot s - \left(\frac{h}{3} + V_o\right) \cdot \left[\gamma_{soil} \cdot b_e \cdot \frac{k_{p_soil}}{FS} \cdot \left(\frac{h^2}{2}\right)\right] + \left(\frac{H}{2} + V_o\right) \cdot q \cdot H \cdot s - P_{p_avg} \cdot b_f \cdot \frac{{V_o}^2}{2} = 544.34 \cdot ft \cdot kips$$

$$F_v := 50$$
ksi  $F_a := .66F_v = 33 \cdot \text{ksi}$ 

$$S_{x_{rqd}} := \frac{M_{max}}{F_{r}} = 197.94 \text{ in}^3$$
 USE W18x106 GR50 PILE



Project Name: _WVARNG South Gate Project No. _7-7728-0000

By: <u>CJR</u> Chk'd By:

#### Precast Lagging Design:

$$w := \frac{P_{a_max}}{H} + q = 950.5 \cdot psf$$

Clear Span of Lagging --->  $S_{clr} := s - 2t_{brg} = 9.17 \, ft$ 

$$S_{chr} := s - 2t_{brg} = 9.17 \, ft$$

$$t_{\text{brg}} := 5 \text{in}$$

$$M_{\text{max_lag}} := w \cdot \frac{S_{\text{clr}}^2 \cdot 1 \, \text{ft}}{10} = 7.99 \cdot \text{ft \cdot kips}$$

$$V_{max_lag} := w \cdot \frac{S_{clr} \cdot 1 \, ft}{2} = 4.36 \cdot kips$$

#### TRY 8" THICK LAGGING

$$f_c := 4ksi$$

$$b := 12in \qquad \qquad t_{lag} := 8in$$

$$f_v := 60 \text{ksi}$$

$$d := t_{lag} - 2in = 6 \cdot in$$

Longitudinal bar

$$d_{bf} := \frac{6}{8} \cdot in$$

$$A_{bf} := \frac{\pi \cdot d_{bf}^2}{4}$$
  $A_{bf} = 0.44 \cdot in^2$ 

$$A_{bf} = 0.44 \text{ in}^2$$

Number of longitudinal bars

Spacing := 
$$\frac{(3ft)}{N}$$
 Spacing = 9·in

Area of flexural steel provided

$$As := \frac{(12in)}{Spacing} A_{bf} \qquad As = 0.59 \cdot in^2$$

$$As = 0.59 \cdot in^2$$

#### MOMENT CALCULATION

$$T_{lag} := As \cdot f_y = 35.34 \cdot kips$$

$$a:=\frac{T_{lag}}{0.85 \cdot f_c \cdot b}=0.87 \cdot in$$

$$M := T_{lag} \cdot \left( d - \frac{a}{2} \right) = 196.75 \cdot in \cdot kips$$

$$M_{\rm u} := M_{\rm max_lag} \cdot \frac{1.7}{0.9} = 181.04 \cdot \text{in} \cdot \text{kips}$$



Project Name: <u>WVARNG South Gate</u>
Road

Project No. <u>7-7728-0000</u>

By: <u>CJR</u> Chk'd By: <u>DWD</u>

#### BALANCED REINFORCEMENT RATIO

$$\rho b := \frac{0.85 \cdot \beta_1 \cdot f_c}{f_v} \cdot \frac{87000 \cdot psi}{87000 \cdot psi + f_v} \qquad \rho b = 0.029$$

Balanced condition

Asmax :=  $0.75 \text{ } \rho \text{b} \cdot \text{b} \cdot \text{d}$  Asmax =  $1.54 \cdot \text{in}^2$ 

ACI 10.3.3 "As" provided for flexure must be less than this to ensure ductile behavior <----

$$\rho min := \frac{200 \cdot psi}{f_y}$$

ACI 10.5 minimum reinforcement required, Alternatively, area of reinforcement provided at every section shall be at least 1/3 greater than that required by analysis. <----

Asmin :=  $\rho \min b \cdot d$  Asmin = 0.24 in²

 $As = 0.59 \cdot in^2$ 

check with above Asmax and Asmin criteria

$$a := \frac{As \cdot f_y}{\beta_1 \cdot f_c \cdot b} \qquad a = 0.87 \cdot in$$

#### SHEAR CHECK

$$V_n := 2\sqrt{f_c} \cdot b \cdot d = 0.13 \text{ ft}^{0.5} \cdot s \cdot lb^{-0.5} \cdot \text{kips}$$

$$V_u := V_{\text{max_lag}} \cdot \frac{1.7}{0.85} = 8.71 \cdot \text{kips}$$

OK!

#### BEARING

$$\begin{split} P_{lag} &:= w \cdot \frac{S_{clr} \cdot 1 ft}{2} = 4.36 \cdot kips \\ A_b &:= \frac{\left(P_{lag} \cdot 1.7\right)}{(0.7) \cdot 0.85 \cdot f_c \cdot 12 in} = 0.26 \cdot in \end{split}$$

MIN. BEARING > .260" PER SIDE, OR .347" WITH A 0.75 FACTOR

FOR 3 FEET TALL PANELS, USE 4-#6 BARS SPACED AT 9" HORIZ., WITH 3" CLEAR AND 4-#4 BARS VERTICAL SPACED EQUALLY ACROSS, WITH 3" CLEAR



Project Name:	20 WVARNG South Gate
Road Project No7-7	728-0000
By: <u>CJR</u>	
Chk'd By: [	)WD

## APPENDIX A SOIL BORING LOGS

AMEC 11003 Bluegrass Parkway Suite 690 Louisville, KY 40299 502-267-0700

BORING	<b>NUMBER E-2</b>	
	D.OF 400F 4	

PAGE 1 OF 1

CLIEN	IT <u>Arn</u>					Gate Roa							
		3MDL( 7.77L0 0000 0002				Camp Daw							
DATE	START	10/10/0				1280.3 ft		HULE	SIZE	3.23			
DRILL	ING C	MIMOLON MATTER	GROUND					3					
DRILL	ING M	ethod <u>HSA</u>				ING							
LOGG	ED BY	MGS				ING <u></u>							
NOTE	S Dry	Hole	AF.	TER DRIL	LING		1	1		ΔΤΤ	ERBE	RG T	
I	9 19 19			SAMPLE TYPE NUMBER	ZD)	BLOW COUNTS (N VALUE)	POCKET PEN. (tsf)	DRY UNIT WT. (pcf)	MOISTURE CONTENT (%)	L	IMITS		FINES CONTENT (%)
DEPTH (#)	GRAPHIC LOG	MATERIAL DESCRIPTION		SAMPL	RECOVERY (RQD)	목요(S) 보일(S)	POCKI	DRY U	MOIS	LIQUID	PLASTIC	PLASTICITY INDEX	FINES (
		NO SAMPLE POSSIBLE DUE TO PRESENCE OF RIP R	AP										
5		GRAVEL, FINE, WITH SILTY CLAY, LOOSE		X ss 1		3-3-4 (7)							
-		ROCK, HIGHLY WEATHERED, SOME SILTY CLAY, DA GRAY	RK	ST 2				100	21	48	28	20	29
10		CLAY, SILTY, WITH FINE TO COARSE ROCK FRAGME SOME WEATHERED SHALE, BROWN, STIFF	ents,	SS 3		2-3-9 (12)							
15				SS 4		22-40-47 (87)							
a lose paragraphs		WEATHERED SHALE, LITTLE SILTY CLAY WITH FINE COARSE SAND, DARK GRAY, VERY DENSE SHALE, LIGHT GRAY	TO	SS 5		50/1"							
20		17.3-17.5, 17.8-18.0 - HIGHLY WEATHERED ZONES 18.6, 19.3, 21.6, 23.5, 24.5 - SLIGHTLY WEATHERED		RC 6	100 (88)					Land Table			
25		FRACTURE 26.4-26.5, 27.0 - MODERATELY WEATHERED FRACTURE 27.3 - SLIGHTLY WEATHERED FRACTURE 27.5 - SLIGHT TO MODERATELY WEATHERED FRAC 28.3-28.5, 28.7, 28.9, 29.5, 29.6, 30.1, 31.1 - SLIGHTLY WEATHERED FRACTURE		RC 7	100 (100)						·		
25									**************************************				
SEOTECH BH COLUMNS - GIN I S I D US LAB. COL				RC 8	92 (50)	WATER-THE PARTY OF THE PARTY OF						L. W. Armon	
SH COLUMNS			The standard and the standards (4	L BC	100				A CONTRACTOR	-			-
Ī.		Refusal at 18.3 feet.				<u></u>	<u> </u>						+
2		Bottom of borehole at 33.7 feet.											

#### **BORING NUMBER E-4** PAGE 1 OF 1

AMEC

11003 Bluegrass Parkway Suite 690 Louisville, KY 40299 502-257-0700

CLIENT Army National Guard	PROJECT NAME South Gate Road Slope Failure
PROJECT NUMBER 7-7728-0000-0002	PROJECT LOCATION Camp Dawson, West Virginia
	GROUND ELEVATION 1273.7 ft HOLE SIZE 3.25
DRILLING CONTRACTOR MATHES	GROUND WATER LEVELS:
DRILLING METHOD HSA	AT TIME OF DRILLING
	AT END OF DOUGHNG

LOGGED BY MGS AFTER DRILLING __ NOTES Dry Hole

			PE	%	_	z.	WT.	111 Se		IMITS		
o DEPTH	GRAPHIC LOG	MATERIAL DESCRIPTION	SAMPLE TYPE NUMBER	RECOVERY (RQD)	BLOW COUNTS (N VALUE	POCKET PEN, (Isf)	DRY UNIT W (pcf)	MOISTURE CONTENT (%)	LIQUID	PLASTIC LIMIT	PLASTICITY INDEX	FINES CONTENT (%)
		GRAVEL, FINE, WITH LITTLE SILTY CLAY AND FINE TO COARSE SAND, LOOSE	X ss 1		4-4-5 (9)							
5_5_		SILT, CLAYEY, LITTLE FINE TO COARSE SAND, LITTLE FINE GRAVEL, TAN AND GRAY, STIFF	SS 2		5-6-9 (15)				referrably a management of the state of the			
		CLAYEY SILT, TAN AND REDDISH BROWN, VERY STIFF	SS 3		15-9-14 (23)							
10		WEATHERED SHALE, DARK GRAY, VERY STIFF	SS 4		4-7-10 (17)						endrenformelendrische Addrig 4 m A	
15		CLAY, SILTY, WITH ROCK FRAGMENTS (HIGHLY WEATHERED), HARD	SS 5		50/3*							

Refusal at 15.0 feet. Bottom of borehole at 15.0 feet.

GEOTECH BH COLUMNS - GINT STD US LAB.GDT - 12/17/09 08:38 - NYGINTPROJECTSVARNG SLOPE FAILURE.GPJ

AMEC
11003 Bluegrass Parkway Suite 690
Louisville, KY 40299
502-267-0700

CLIENT A	my National Guard	PROJEC	TNAME	South	Gate Roa	d Slop	e Faik	ıre				
1	UMBER 7-7728-0000-0002				Camp Daw							
DATE STAF	TED 10/9/08 COMPLETED 10/9/08	GROUND	ELEVAT		1271.2 ft		HOLE	SIZE	3.25			
DRILLING O	ONTRACTOR MATHES	GROUND										
DRILLING R	ETHOD HSA				JNG							
LOGGED B	Y MGS				ING							
NOTES D	y Hote	AF	TER DR!	LUNG						FDDE		
			YPE 3	%	တယ်	Ë	WT.	일 (%	All	ERBE IMITS		TENT
GRAPHIC LOG	MATERIAL DESCRIPTION		SAMPLE TYPE NUMBER	RECOVERY 9 (RQD)	BLOW COUNTS (N VALUE)	POCKET PEN. (tsf)	DRY UNIT WT. (pcf)	MOISTURE CONTENT (%)	LIQUID	PLASTIC LIMIT	PLASTICITY INDEX	FINES CONTENT (%)
	SILT, CLAYEY, MIX OF GRAY AND REDDISH BROWN MEDIUM STIFF		Ss 1		4-4-3 (7)			,				
5 , 9	SILT, CLAYEY, TRACE FINE GRAVEL, LITTLE BLACK NODULES, BROWN, MEDIUM STIFF		SS 2		5-4-4 (8)							
	SILT, CLAYEY, TRACE BLACK NODULES, TAN, VERY	STIFF	X SS 3		6-11-11 (22)							
10			SS 4		8-9-11 (20)							
G SLOPE FAILURE GPJ	SILT, CLAYEY, LITTLE FINE TO COARSE ROCK FRAG TAN, VERY STIFF	MENTS,	SS 5		15-12-12 (24)	And the second s	THE					
N.GINTPROJECTSVARN	SHALE, LIGHT GRAY  18.7, 19.6 - SLIGHTLY WEATHERED FRACTURE 20.0 - SLIGHT TO MODERATELY WEATHERED FRACT 20.3-21.2, 21.7-21.9 - ROCK FRAGMENTS, SLIGHT TO MODERATELY WEATHERED 22-23 ZONE OF LOSS 23-23.5 ROCK FRAGMENTS SLIGHT TO MODERATELY		RC 6	100 (72)		77.6.5.00.800.00.000.000.000.000.000.000.000			And the state of t			
GEOTECH BH COLUMNS - GINT 5TD US LAB GDT - 12/17/08 08:41 - N;GINTPROJECTSARING SLOPE FAILURE GRU  COLUMNS - GINT 5TD US LAB GDT - 12/17/08 08:41 - N;GINTPROJECTSARING SLOPE FAILURE GRU  COLUMNS - GINT 5TD US LAB GDT - 12/17/08 08:41 - N;GINTPROJECTSARING SLOPE FAILURE GRU  COLUMNS - GINT 5TD US LAB GDT - 12/17/08 08:41 - N;GINTPROJECTSARING SLOPE FAILURE GRU  COLUMNS - GINT 5TD US LAB GDT - 12/17/08 08:41 - N;GINTPROJECTSARING SLOPE FAILURE GRU  COLUMNS - GINT 5TD US LAB GDT - 12/17/08 08:41 - N;GINTPROJECTSARING SLOPE FAILURE GRU  COLUMNS - GINT 5TD US LAB GDT - 12/17/08 08:41 - N;GINTPROJECTSARING SLOPE FAILURE GRU  COLUMNS - GINT 5TD US LAB GDT - 12/17/08 08:41 - N;GINTPROJECTSARING SLOPE FAILURE GRU  COLUMNS - GINT 5TD US LAB GDT - 12/17/08 08:41 - N;GINTPROJECTSARING SLOPE FAILURE GRU  COLUMNS - GINT 5TD US LAB GDT - 12/17/08 08:41 - N;GINTPROJECTSARING SLOPE FAILURE GRU  COLUMNS - GINT 5TD US LAB GDT - 12/17/08 08:41 - N;GINTPROJECTSARING SLOPE FAILURE GRU  COLUMNS - GINT 5TD US LAB GDT - 12/17/08 08:41 - N;GINTPROJECTSARING SLOPE FAILURE GRU  COLUMNS - GINT 5TD US LAB GDT - 12/17/08 08:41 - N;GINTPROJECTSARING SLOPE FAILURE GRU  COLUMNS - GINT 5TD US LAB GDT - 12/17/08 08:41 - N;GINTPROJECTSARING SLOPE FAILURE GRU  COLUMNS - GINT 5TD US LAB GDT - 12/17/08 08:41 - N;GINTPROJECTSARING SLOPE FAILURE GRU  COLUMNS - GINT 5TD US LAB GDT - 12/17/08 08:41 - N;GINTPROJECTSARING SLOPE FAILURE GRU  COLUMNS - GINT 5TD US LAB GDT - 12/17/08 08:41 - N;GINTPROJECTSARING SLOPE FAILURE GRU  COLUMNS - GINT 5TD US LAB GDT - 12/17/08 08:41 - N;GINT - 12/17/08 08:41 - N;GINTPROJECTSARING SLOPE FAILURE GDT - 12/17/08 08:41 - N;GINT - 12/17/08	WEATHERED 23.8 MODERATELY WEATHERED FRACTURE 24.3-24.5 ROCK FRAGMENTS SLIGHT TO MODERATE WEATHERED 24.8, 24.9 MODERATELY WEATHERED FRACTURE 25.4, 25.8, 25.9, 26.2 - SLIGHT TO MODERATELY WEATHERED FRACTURE 28.1, 29.4, 29.9, 30.3 - SLIGHTLY WEATHERED FRACTURE 31.1, 31.4 - SLIGHT TO MODERATELY WEATHERED	THERED	RC 7	70 (44)	d'aussiere remaine de l'action							
OLUMNS - GINT STD US	FRACTURE 30.8-31.0 - VERTICAL FRACTURE		RC 8	92 (82)			}					
ЗЕОТЕСН ВН С	Refusal at 17.0 feet. Bottom of borehole at 32.0 feet.											

PROJECT NAME   South Care   Project NUMBER   7:772-8009-0022   PROJECT LOCATION   Care   Dayson, West Virginia   PROJECT LOCATION   Care   Dayson, West Virginia   PROJECT LOCATION   Care   Dayson, West Virginia   PROJECT LOCATION   L271 ii   HOLE SIZE   3:25   DRILLING CONTRACTOR   MATHES   DRILLING METHOD   HAS   AT TIME OF DRILLING   AT TIME OF D	am	eo	AMEC 11003 Bluegrass Parkway Suite 690 Louisville, KY 40299 502-267-0700							G N		PAGE		
PROJECT NUMBER 7-7728-0000-0002  DATE STARTED 10/9/08 COMPLETED 10/9/08  DRILLING CONTRACTOR MATHES  DRILLING CONTRACTOR MATHES  DRILLING METHOD HSA  LOGGED BY MGS  NOTES Dry Hole  MATERIAL DESCRIPTION  CLAY, SILTY, LITTLE BLACK NODULES, MIX OF TAN, REDDISH BROWN AND GRAY, STIFF  CLAY, SILTY, LITTLE BLACK NODULES, MIX OF TAN, REDDISH BROWN AND GRAY, STIFF  CLAY, SILTY, WITH FINE GRAINED SAND, REDDISH BROWN AND GRAY  SILT, CLAYEY, WITH FINE GRAINED SAND, TAN, STIFF  CLAY, SILTY, TAN AND REDDISH BROWN, VERY STIFF  SILT, CLAYEY, WITH FINE GRAINED SAND, TAN, STIFF  AT TIME OF DRILLING —  ATTERBERG CLIMITS  ATTERBERG C	CLIEN	IT Arπ	14 14000101 C2010											
DATE STARTED 10/9/08 COMPLETED 10/9/08  DRILLING CONTRACTOR MATHES  DRILLING METHOD HSA  LOGGED BY MGS  NOTES Dry Hole  MATERIAL DESCRIPTION  O  CLAY, SILTY, LITTLE BLACK NODULES, MIX OF TAN, REDDISH BROWN AND GRAY, STIFF  CLAY, SILTY, TAN AND REDDISH BROWN, VERY STIFF  10  DATE STARTED 10/9/08  GROUND WATER LEVELS:  GROUND WATER LEVELS:  AT TIME OF DRILLING —  ATTERBERG LIMITS  ATTERBER	1		IMBER 7-7728-0000-0002			******								
DRILLING METHOD HSA  LOGGED BY MGS  NOTES Dry Hole  MATERIAL DESCRIPTION  O  CLAY, SILTY, LITTLE BLACK NODULES, MIX OF TAN, REDDISH BROWN AND GRAY, STIFF  OCLAY, SILTY, WITH FINE GRAINED SAND, REDDISH BROWN AND GRAY  SILT, CLAYEY, WITH FINE GRAINED SAND, TAN, STIFF  10  SILT, CLAYEY, WITH FINE GRAINED SAND, TAN, STIFF  SILT, CLAYEY, WITH FINE GRAINED SAND, TAN, STIFF  15  SILT, CLAYEY, WITH FINE GRAINED SAND, TAN, STIFF	DATE	START	ED 10/9/08 COMPLETED 10/9/08						HOLE	SIZE	3.25			
DRILLING METHOD HSA  LOGGED BY MGS  NOTES Dry Hole  MATERIAL DESCRIPTION  MATERIAL DESCRIPTION  MATERIAL DESCRIPTION  O  CLAY, SILTY, LITTLE BLACK NODULES, MIX OF TAN, REDDISH BROWN AND GRAY, STIFF  CLAY, SILTY, WITH FINE GRAINED SAND, REDDISH BROWN AND GRAY  CLAY, SILTY, WITH FINE GRAINED SAND, REDDISH BROWN AND GRAY  SS 4  SS 4  SS 6-6-6  (12)  SS 6-6-6  (12)  SS 1  SS 6-6-6-6  (12)  SS 1  SS 6-6-6-6  (12)  SS 1  SS 6-6-6-6  (12)	DRILL	ING CO	NTRACTOR MATHES	GROUND	WATER	LEVEL	S:							
LOGGED BY MGS  NOTES Dry Hole  AFTER DRILLING  HE WAS AFTER DRILLING  HE WAS AFTER DRILLING  HE WAS AFTER DRILLING  MATERIAL DESCRIPTION  O  CLAY, SILTY, LITTLE BLACK NODULES, MIX OF TAN, REDDISH BROWN AND GRAY, STIFF  CLAY, SILTY, LITTLE BLACK NODULES, MIX OF TAN, REDDISH BROWN AND GRAY, STIFF  SILT, CLAYEY, TRACE FINE GRAVEL, TRACE ORGANICS, TAN, STIFF  CLAY, SILTY, WITH FINE GRAINED SAND, REDDISH BROWN AND GRAY  TOLAY, SILTY, WITH FINE GRAINED SAND, REDDISH BROWN AND GRAY  SHAPP OF THE WAS AFTER DRILLING  TO AFTER DRILLING  ATTERBERG LIMITS  ATTERBERG LIMITS  ATTERBERG  LIMITS  ATTERBERG  LIMITS  ATTERBERG  LIMITS  ATTERBERG  LIMITS  AND AND AND LIMITS  ATTERBERG  LIMITS  AND AND AND LIMITS  ATTERBERG  LIMITS  AND AND AND LIMITS  ATTERBERG  LIMITS  AND AND LIMITS  AND AND AND LIMITS  ATTERBERG  LIMITS  AND AND LIMITS  AND AND LIMITS  ATTERBERG  LIMITS  AND AND LIMITS  AND AND LIMITS  ATTERBERG  LIMITS  AND LIMITS  AND AND LIMITS  AND AND LIMITS  AND AND AND LIMITS  AND				AT	TIME OF	DRILL	ing							
NOTES DIY Hole  AFTER DRILLING  H. G. DIY Hole  MATERIAL DESCRIPTION  MATERIAL DESCRIPTI	1			ΑT	END OF	DRILLI	NG							
ATTERBERG LIMITS  AND				AF	TER DRII	_LING								
CLAY, SILTY, LITTLE BLACK NODULES, MIX OF TAN, REDDISH BROWN AND GRAY, STIFF  SILT, CLAYEY, TRACE FINE GRAVEL, TRACE ORGANICS, TAN, STIFF  CLAY, SILTY, WITH FINE GRAINED SAND, REDDISH BROWN AND GRAY  CLAY, SILTY, TAN AND REDDISH BROWN, VERY STIFF  SS 4  10  SS 5-5-6 (11)  SS 2  11  112  19  39  22  17  SILT, CLAYEY, WITH FINE GRAINED SAND, TAN, STIFF  SS 6-6-6 (12)					SAMPLE TYPE NUMBER	RECOVERY % (RQD)	BLOW COUNTS (N VALUE)	POCKET PEN. (tsf)	DRY UNIT WT. (pcf)	MOISTURE CONTENT (%)		IMITS	}	FINES CONTENT (%)
TAN, STIFF  CLAY, SILTY, WITH FINE GRAINED SAND, REDDISH BROWN AND GRAY  CLAY, SILTY, TAN AND REDDISH BROWN, VERY STIFF  CLAY, SILTY, TAN AND REDDISH BROWN, VERY STIFF  SILT, CLAYEY, WITH FINE GRAINED SAND, TAN, STIFF  15			REDDISH BROWN AND GRAY, STIFF											
AND GRAY  CLAY, SILTY, TAN AND REDDISH BROWN, VERY STIFF  SS 4  10-12-15 (27)  SILT, CLAYEY, WITH FINE GRAINED SAND, TAN, STIFF  SS 6-6-6 (12)  15	5		TAN, STIFF				"							- Landana Maria
SILT, CLAYEY, WITH FINE GRAINED SAND, TAN, STIFF  SS 6-6-6 (12)  15	-		AND GRAY						112	19	39	22	17	91
SS 6-6-6 (12) 15	10			•						Aprilia de la companya de la company		- things of the second of the		
CLAY, SILTY, SOME FINE ROCK FRAGMENTS, MIX OF TAN AND DARK GRAY, STIFF  SS 8-7-7 (14)  WEATHERED SHALE, VERY DENSE  RC 85 (71)  RC 93 9 (76)	OPE FAILUHE.GPU		SILT, CLAYEY, WITH FINE GRAINED SAND, TAN, STIF	<b>:</b>	SS 5		i			15	The state of the s	ALL PROPERTY OF THE PROPERTY O		54
### DELIGITION - STATE - 1	OJECTSVARNG SI		CLAY, SILTY, SOME FINE ROCK FRAGMENTS, MIX OF AND DARK GRAY, STIFF	F TAN	X ss				- PARAMETERS	10				58
SS 7 46-50/1"  RC 85 (71)  RC 93 9 (76)	18:45 - NAGINTAP		WEATHERED SHALE, VERY DENSE											7.000
BC 85 8 (71)  RC 93 93 99 (76)	25					_	46-50/1	÷	1-1-10-10-10-10-10-10-10-10-10-10-10-10-					-
BC 93 (76) 9 (76)	D US LAB.GD					85 (71)								
DE COLUMN TO THE PROPERTY OF T	UMNS - GINT ST						***************************************	To the state of th	Maria and a second distance of the second dis			- Contract	***************************************	
	OTECH BH COL			migrae and accept accept accept accept										

(Continued Next Page)

AMEC

11003 Bluegrass Parkway Suite 690 Louisville, KY 40299

502-267-0700

**BORING NUMBER E-6** 

PAGE 2 OF 2

PROJECT NAME South Gate Road Slope Failure PROJECT LOCATION Camp Dawson, West Virginia

100

(61)

RC

11

PROJECT NUMBER _7-7728-0000-0002

CLIENT Army National Guard

EN ATTERBERG SAMPLE TYPE NUMBER MOISTURE CONTENT (%) LIMITS POCKET PEN. (Isl) ≶ BLOW COUNTS (N VALUE) FINES CONT PLASTICITY INDEX RECOVERY (RQD) E (S PLASTIC LIMIT GRAPHIC LOG LIQUID DEPTH (II) MATERIAL DESCRIPTION PRY SHALE, LIGHT GRAY 10 (85)

35

28.4 - MODERATELY TO HIGHLY WEATHERED FRACTURE 28.9 - SLIGHTLY WEATHERED FRACTURE

29.2, 29.4 - MODERATELY WEATHERED FRACTURE 29.8-30.3 - VERTICAL FRACTURE, SLIGHTLY WEATHERED 30.5 - MODERATELY WEATHERED FRACTURE

30.7 - SLIGHTLY WEATHERED FRACTURE 30.5-30.7 - VERTICAL FRACTURE

31.8 - SLIGHTLY WEATHERED FRACTURE 33.0 - SLIGHTLY WEATHERED FRACTURE

33.5, 34.9 - MODERATELY WEATHERED FRACTURE 35.2, 36.8-36.9, 37.2, 38.2 - SLIGHTLY WEATHERED

FRACTURE 38.9 - MODERATELY WEATHERED FRACTURE
39.3 - SLIGHT TO MODERATELY WEATHERED

40.1, 40.7 - SLIGHTLY WEATHERED FRACTURE (continued from previous page)

Refusal at 25.0 feet. Bottom of borehole at 40.9 feet.

COLUMNS - GINT STD US LAB.GDT - (2/17/08 08:45 - N.)GINTYPROJECTSVARNG SLOPE FAILURE.GPJ

표 GEOTECH

AIMEC 11003 Bluegrass Parkway Suite 690 Louisville, KY 40299 502-257-0700

302-207 0700	•
CLIENT Army National Guard	PROJECT NAME South Gate Road Slope Failure
PROJECT NUMBER 7-7728-0000-0002	PROJECT LOCATION Camp Dawson, West Virginia
DATE STARTED 10/8/08 COMPLETED 10/8/08	GROUND ELEVATION 1266 ff HOLE SIZE 3.25
DATE OF A STATE OF A S	GROUND WATER LEVELS:
	AT TIME OF DRILLING
DRILLING METHOD HSA	AT END OF DRILLING
LOGGED BY MGS	AFTER DRILLING —
NOTES Dry Hole	A TENEDO I.

		1 1/10	П П	%	_	z	۷T.	ار ا%)		ERBE IMITS		ENT
	GRAPHIC LOG	MATERIAL DESCRIPTION	SAMPLE TYPE NUMBER	RECOVERY (RQD)	BLOW COUNTS (N VALUE)	POCKET PEN. (tsf)	DRY UNIT WT. (pcf)	MOISTURE CONTENT (%)	LIGUID	PLASTIC LIMIT	PLASTICITY INDEX	FINES CONTENT
0 ]		CLAY, SILTY, SOME FINE TO COARSE SAND, TRACE FINE GRAVEL, BROWN, MEDIUM STIFF	X ss		3-3-4 (7)							
 		CLAY, SILTY, SOME FINE TO COARSE SAND, TRACE FINE GRAVEL, BROWN, MEDIUM STIFF	SS 2		3-3-3 (6)			***************************************				
-		CLAY, SILTY, SOME FINE TO COARSE SAND, BROWN, VERY STIFF	SS 3		8-6-10 (16)	1		15				53
10		CLAY, SILTY, SOME FINE TO COARSE SAND, BROWN, STIFF	SS 4		4-7-8 (15)			15	1			65
15		SILT, CLAYEY, TRACE FINE TO COARSE ROCK FRAGMENTS, BROWN, HARD	SS 5		6-8-28 (36)			THE	- Andrews - Andr	entition and an article and article article and article and article article and article article article and article article article article and article articl	A PRINCIPAL DE LA CALLANTINE DE LA CALLA	Administration of the state of
- ·		SILT, CLAYEY, DARK GRAY, STIFF	SS 6		6-8-7 (15)			15				5
20_		Refusal at 21.0 feet.				]	<u></u>		]			-
15		Bottom of borehole at 21.0 feet.										
	12. ****		ename or to, pure or makes	majo madele propinsi anno						N. W. T. Variable (1997)	744 Tes	-

LIENT Army Nation ROJECT NUMBER ATE STARTED 10 RILLING CONTRAC	7-7728-0000-0002 //8/08	PROJEC	FLOCATI ELEVAT	ON C	amp Daws	on, W	Failu est Vi	re ginia				
ROJECT NUMBER ATE STARTED 10 RILLING CONTRAC RILLING METHOD OGGED BY MGS	7-7728-0000-0002 0/8/08	PROJEC*	FLOCATI ELEVAT	ON C	amp Daws	on, W	est Vi	ginia				
ATE STARTED 10 RILLING CONTRAC RILLING METHOD OGGED BY MGS	%8/08 COMPLETED 10/8/08 CTOR MATHES			ION 1	000 4 2							
RILLING CONTRAC RILLING METHOD OGGED BY <u>MGS</u>	CTOR MATHES	GROUND			∠66.4 <del>11</del>	ŀ	OLE :	SIZE	3.25			
OGGED BY MGS	HSA		WATER	LEVEL	S:							
		AT TIME OF DRILLINGAT END OF DRILLING										
OTES <u>Dry Hole</u>	· .											
		AF	TER DRIL	LING				T	ATT	ERBE		
GRAPHIC LOG	MATERIAL DESCRIPTION	<b>✓</b>	SAMPLE TYPE NUMBER	RECOVERY % (RQD)	BLOW COUNTS (N VALUE)	POCKET PEN. (tsf)	DRY UNIT WT. (pcf)	MOISTURE CONTENT (%)	LIQUID	IMITS		
O CLA BRC	Y, SILTY, LITTLE FINE TO COARSE GRAVEL, RE OWN AND GRAY, VERY STIFF	EDDISH	X SS 1		8-9-11 (20)							
5	Y, SILTY, TRACE FINE GRAVEL, TAN, STIFF		SS 2		9-5-7 (12)							
SAN GR/	Y, SILTY, LITTLE ORGANICS, LITTLE FINE TO CO ID, TRACE FINE GRAVEL, MIX OF REDDISH BRO AY, STIFF	MAM WIAD	SS 3		3-6-6 (12)							
10 NOI	Y, SILTY, LITTLE FINE TO COARSE SAND, LITTL DULES, MIX OF GRAY AND BROWN, STIFF		SS 4		3-5-5 (10)							
LIT	Y, SILTY, SOME FINE TO COARSE ROCK FRAGM TLE BLACK NODULES, BROWN AND REDDISH BI DIUM STIFF	WENTS, ROWN,	X ss 5		4-3-3 (6)				The state of the s			
SIL.	T, CLAYEY, BROWN AND REDDISH BROWN, STI	FF	1 68		3-4-6							
20			SS 6		(10)			16				
SH	ALE											
	Refusal at 23.5 feet. Bottom of borehole at 23.5 feet.		SS 7	<del></del>	50/1*	<i></i>						

aneco

AMEC 11003 Bluegrass Parkway Suite 690

	att		Louisville, KY 40299 502-267-0700											
	CLIEN	ET Am	ry National Guard	PROJECT	NAME .	South	Gate Roa	d Slop	e Failu	ıre		···-		
			IMBER 7-7728-0000-0002	PROJECT	LOCAT	מסו	amp Daw	son, W	est Vi	rginia				
			ED 10/8/08 COMPLETED 10/8/08	GROUND	ELEVAT	TON _	266.5 ft	1	HOLE	SIZE	3.25			
			NTRACTOR MATHES											
			THOD HSA				ING _ <del>_</del> _							
-	LOGG	ED BY	MGS	AT I	END OF	DRILLI	NG							
	NOTE	S Dry	Hole	AFT	ER DRIL	LING								
					ា	%		÷	Т.	(9)	ATT	ERBE IMITS	RG	E
	DEPTH (II)	GRAPHIC LOG	MATERIAL DESCRIPTION	- The state of the	SAMPLE TYPE NUMBER	RECOVERY (RQD)	BLOW COUNTS (N VALUE)	POCKET PEN. (tsf)	DRY UNIT W (pcf)	MOISTURE CONTENT (%)	LIQUID		PLASTICITY INDEX	FINES CONTENT (%)
	<u> </u>		SILT, CLAYEY, WITH FINE TO COARSE GRAVEL, SON BLACK NODULE, BROWN, VERY STIFF		∬ ss 1		6-9-14 (23)							
	 - <u>5</u>		SAND, SILTY, TRACE FINE TO COARSE SAND, TRAC GRAVEL, TRACE ORGANICS, BROWN, STIFF		SS 2		6-6-8 (14)	1		16				47
			CLAY, SILTY, LITTLE FINE TO COARSE SAND, TRAC BLACK NODULE, GRAY, MEDIUM STIFF		SS 3		4-2-3 (5)			15	38	23	15	
	10		SILT, CLAYEY, TRACE FINE GRAVEL, BROWN, MEDII STIFF		SS 4		3-3-4 (7)		- Control of the Cont					
CTSVARNG SLOPE FAILURE.GPJ	15		CLAY, SILTY, SOME FINE TO COARSE SAND, LITTLE NODULES, BROWN, STIFF		SS 5		2-4-6 (10)					The second secon		
NYGINTYPROJECTSVARN	20		SILT, CLAYEY, LITTLE WEATHERED SHALE, BROWN TAN, VERY STIFF		SS 6		6-8-10 (18)	The state of the s						
GEOTECH BH COLUMNS - GINT STD US LAB.GDT - 12/17/08 08:46 - N.\GINT\PROJE	25		SILT, CLAYEY, WITH FINE GRAINED SAND, REDDISH BROWN, MEDIUM STIFF	1	SS 7		3-2-4 (6)							
COLUMNS - GINT STD US	30				R 8	97 (20)			a market a destruction of the state of the s		AAAAAAAAAAAAAAAAAAAAAAAAAAAAAAAAAAAAAA		***************************************	
GEOTECH BH (	35				RC 9	91 (51)							uniformitation of a V. a a -	Talente Table

AMEC 11003 Bluegrass Parkway Suite 690 Louisville, KY 40299

502-267-0700

CLIENT Army National Guard

PROJECT NAME South Gate Road Slope Failure

BORING NUMBER E-9
PAGE 2 OF 26

ATTERBERG LIMITS

MOISTURE CONTENT (%)
LIQUID
LIMIT
PLASTIC
LIMIT
SATICITY
INDEX

POCKET PEN. (Isf)
DRY UNIT WT. (pcf)

FINES CONTENT (%)

PROJECT LOCATION Camp Dawson, West Virginia PROJECT NUMBER 7-7728-0000-0002

L			ļ			ĺ
DEPTH (ft)	GRAPHIC LOG	MATERIAL DESCRIPTION	10 t	NUMBER	RECOVERY % (RQD)	WO IB
35		SHALE, LIGHT GRAY	П			Γ
40		27.95, 28.15, 28.5, 29, 29.1, 29.3, 29.5 - MODERATE TO HIGHLY WEATHERED FRACTURE 29.7, 29.8, 30.0 - SLIGHT TO MODERATELY WEATHERED FRACTURE 30.1, 30.2, 30.4 - HIGHLY WEATHERED FRACTURE 31.1, 31.4 - SLIGHTLY WEATHERED FRACTURE 31.6, 31.7 - MODERATELY WEATHERED FRACTURE		RC 10	100 (54)	
		31.9, 32.1 - HIGHLY WEATHERED FRACTURE 32.7 - SLIGHTLY WEATHERED FRACTURE 32.4-23.5 - HIGHLY WEATHERED ZONE OF LOSS		RC 11	100 (50)	
		3.9, 34.0 - MODERATELY WEATHERED FRACTURE 34.5 - SLIGHTLY WEATHERED FRACTURE 34.7 - MODERATELY WEATHERED FRACTURE 36.5 - SLIGHTLY WEATHERED FRACTURE 38.9, 37.6, 38.1, 38.3, 38.4, 38.6 - MODERATELY WEATHERED FRACTURE 39, 39.3, 39.4, 39.5, 39.6, 39.8 - SLIGHT TO MODERATELY WEATHERED 40.7, 40.9 - 41.0 - MODERATELY WEATHERED FRACTURE 41.5, 41.7, 42.0 - MODERATE TO HIGHLY WEATHERED FRACTURE (continued from previous page)  Befusal at 27.5 feet.				

Refusal at 27.5 feet. Bottom of borehole at 42.5 feet.

GEOTECH BH, COLUMNS - GINT 810 US LAB.GDT - 12/17/08 08:46 - NAGINTAPROJECTSVARNG SLOPE FAILURE.GPJ

AMEC

an		Louisville, KY 40299 502-267-0700														
CLIEN	T Am	ny National Guard F	PROJECT	NAME_	South	Gate Roa	d Slop	e Failu	ire		·		[			
PROJECT NUMBER 7-7728-0000-0002 PROJECT LOCATION Camp Daws								· · · · · · · · · · · · · · · · · · ·								
DATE	DATE STARTED 10/7/08 COMPLETED 10/7/08 GROUND ELEVATION 1268.8 ft HOLE SIZE 3.25															
DRILL	ING CO	ONTRACTOR MATHES	ROUND I	NATER I	LEVEL	S:							}			
		ETHOD HSA	AT T	TME OF	DRILL	ing										
į.		MGS	AT E	ND OF I	DRILLI	NG										
NOTE	S <u>Dry</u>	Hole	AFT	ER DRIL	LING		,				ERBE					
				SAMPLE TYPE NUMBER	%	(C)	ż	DRY UNIT WT. (pcf)	ш <u>®</u>	AII	IMITS	nu	FINES CONTENT (%)			
Ţ	GRAPHIC LOG				田口	BLOW COUNTS (N VALUE)	POCKET PEN. ((sf)	Ē	MOISTURE CONTENT (%)	۱۵.	۵. ا	ĔŢ	2			
DEPTH (ft)	AP O	MATERIAL DESCRIPTION		를 를 들는	호텔	₩ ₹	문	j j	SE	LIGUID	PLASTIC LIMIT	FE B	၁			
0	P.		į	SAN	RECOVERY (RQD)	ے	Ö.	РН	<b>≥</b> 8	5-	7-	PLASTICITY INDEX				
0		GRAVEL, FINE TO COARSE, SOME SILTY CLAY, MEDIL	IM				-									
-		DENSE		// ss	ļ	6-6-5	1									
-			2	1	1	(11)										
		CLAY, SILTY, SOME FINE TO COARSE SAND, BROWN,			ļ											
-		MEDIUM STIFF		X SS 2		2-3-5 (8)										
5				V -			1									
-		CLAY, SILTY, SOME FINE TO COARSE SAND, BROWN	AND	√l ss		1-2-2	1	ĺ			Ì					
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(Continued Next Page)

CLIENT Army National Guard

PROJECT NUMBER 7-7728-0000-0002

AMEC 11003 Bluegrass Parkway Suite 690

Louisville, KY 40299 502-267-0700

PROJECT NAME South Gate Road Slope Failure

RECOVERY (RQD)

100

SAMPLE TYPE NUMBER

SS

9 (100)

PROJECT LOCATION Camp Dawson, West Virginia

BLOW COUNTS (N VALUE)

POCKET PEN. (tsf)

Cod)

DRY

**BORING NUMBER E-10** 

MOISTURE CONTENT (%)

PAGE 2"OF 2"

FINES CONTENT (%)

PLASTICITY INDEX

**ATTERBERG** 

LIMITS

PLASTIC LIMIT

LIQUID

GRAPHIC LOG DEPTH MATERIAL DESCRIPTION SHALE, LIGHT GRAY 27.4- SLIGHTLY WEATHERED FRACTURE 27.5 - MODERATE TO HIGHLY WEATHERED FRACTURE 27.8 - SLIGHTLY WEATHERED FRACTURE 27.9 - HIGHLY WEATHERED, MUD SEAM 28.1 - HIGHLY WEATHERED, MUD SEAM 28.4-28.6 - HIGHLY WEATHERED, MUD SEAM 28.9, 29.0 - MODERATE TO HIGHLY WEATHERED FRACTURE 29.2 - SLIGHTLY WEATHERED FRACTURE 29.4, 29.9 - MODERATE TO HIGHLY WEATHERED FRACTURE 30.2, 30.4 - SLIGHTLY WEATHERED FRACTURE 30.7 - SLIGHT TO MODERATELY WEATHERED FRACTURE 30.7-30.9 - SLIGHTLY WEATHERED FRACTURE 31.0, 31.1, 31.4, 31.6 - HIGHLY WEATHERED, MUD SEAM 32.2, 32.4, 32.7 - MODERATELY WEATHERED FRACTURE 33.0, 33.1, 33.3 - SLIGHTLY WEATHERED FRACTURE 33.5, 33.8, 33.9, 34.1, 34.4, 34.5, 34.6- MODERATELY WEATHERED FRACTURE 34.9 - SLIGHTLY WEATHERED FRACTURE 35.44, 35.7, 35.8 -MODERATELY WEATHERED FRACTURE (continued from previous page)

Refusal at 27.0 feet. Bottom of borehole at 39.0 feet.

COLUMNS - GINT STO US LAB.GDT - 12/17/08 08:34 - NAGINTYPROJECTS/ARNG SLOPE FAILURE.GPJ

H GEOTECH

AMEC

ati		11003 Bluegrass Parkway Suite 690 Louisville, KY 40299 502-267-0700						٠					
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		IMBER _7-7728-0000-0002 F	ROJEC	T LOCAT	ION _	Damp Daw	son, W	est Vi	rginia				
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DEPTH (ft)	GRAPHIC	MATERIAL DESCRIPTION		SAMPLE TYPE NUMBER	RECOVERY ° (RQD)	BLOW COUNTS (N VALUE)	POCKET PEN. (Isf)	DRY UNIT WT. (pcf)	MOISTURE CONTENT (%)			PLASTICITY INDEX	FINES CONTENT (%)
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  -		BROWN, MEDIUM STIFF (PROBABLE FILL)		SS 1		5-4-3 (7)	1						
 - 5		CLAY, SILTY, WITH FINE TO COARSE GRAINED SAND, AND BROWN, MEDIUM STIFF (PROBABLE FILL)	GRAY	X SS 2		3-3-4 (7)							
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15		CLAY, SILTY, LITTLE FINE TO COARSE SAND, REDDIS BROWN, STIFF	ਜ <i></i>	X ss 5		4-4-5 (9)	-		20	41	23	18	
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Refusal at 27.0 feet. Bottom of borehole at 27.0 feet.													
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GEOTECH BH

# BORING NUMBER E-12 PAGE 1 OF 1 6-6-5 (11)8-4-3 (7) 0-2-4 (6)

anec
anec

**AMEC** 

NO RECOVERY

CLAY, SILTY, SOME FINE TO COARSE GRAINED SAND, TAN, MOIST, STIFF

Refusal at 28.2 feet. Bottom of borehole at 28.2 feet.

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DEPTH (f)	GRAPHIC GRAPHIC LOG	MATERIAL DESCRIPTION		SAMPLE TYPE NUMBER	RECOVERY % (RQD)	BLOW COUNTS (N VALUE)	POCKET PEN. (tsf)	DRY UNIT WT. (pcf)	MOISTURE CONTENT (%)		PLASTIC IMIT	PLASTICITY BA	FINES CONTENT (%)
<u>0</u>	, , , , , , , , , , , , , , , , , , ,	SILT, CLAYEY, SOME FINE TO COARSE GRAVEL, LIT FINE TO COARSE SAND, TAN, STIFF, DRY SILT, CLAYEY, SOME FINE TO COARSE SAND, TAN, M		SS 1		6-7-6 (13) 6-5-6							
ļ		STIFF	•	SS 2		(11)							

SS

3

SS

10 CLAY, SILTY, TAN AND REDDISH BROWN, SOME FINE TO COARSE SAND, LITTLE FINE TO COARSE ROCK FRAGMENTS, MEDIUM STIFF COLUMNS - GINT STD US LAB.GDT - 12/17/08 08:35 - NYGINTPROJECTSVARNG SLOPE FAILURE.GPJ SS 5 CLAY, VERY SILTY, REDDISH BROWN, SOME FINE TO COARSE GRAINED SAND, LITTLE FINE TO COARSE ROCK FRAGMENTS, MOIST, STIFF SS 6 4-5-5 (10) CLAY, SILTY, BROWN AND TAN, SOME FINE TO COARSE SAND, LITTLE BLACK NODULES, MOIST, HARD SS 7 9-15-16 (31)

Y	MILS (mm)
В	79 (2.0 mm)
С	109 (2.7 mm)
D	138 (3.5 mm)
Е	168 (4.3 mm)
F	188 (4.8 mm)
G	218 (5.5 mm)
Н	249 (6.3 mm)
J	280 (7.1 mm)

#### 613-BLANK

### SECTION 614 PILING WALLS

#### 614.1 - DESCRIPTION:

This work shall consist of furnishing and placing steel piles in predrilled holes, concrete or grout, backfill and lagging, of the kinds and dimensions designated, in accordance with these provisions and in reasonably close conformity with the lines, grades, dimensions, and locations shown on the Plans or established by the Engineer. Painting of the exposed steel is included.

Careful attention shall be given to assuring the pile wall will tie directly into an existing stable slope. Prior to ordering any materials, the contractor in conjunction with the Engineer shall conduct a project site review in order to verify the limits of the pile wall.

#### 614.2 - MATERIALS:

Materials shall conform to the requirements specified in the following Subsections of Division 700:

MATERIAL	SUBSECTION
Steel Piles and Splices	709.12
Steel Lagging and Wales	709.12
Reinforcing Steel	709.1
Prestressing Steel	709.2
Treated Timber Lagging	710
Portland Cement	701.1
Fine Aggregate	702.1
Fly Ash	707.4

#### 614.3 - DRILLING:

614.4

A drilled hole is required for the buried length of the pile.

A minimum of 1/3 the total pile length or 10 feet (3 m), whichever is greater, is to be placed in bedrock/shale. Deviation from this requirement will be controlled by a Plan note. The total estimated pile length and the depth to the estimated bedrock/shale line are shown on the piling profile. Should the elevation of the actual bedrock/shale vary from the estimated elevation by more than 2.5 feet (0.8 m), the Engineer must approve the hole prior to placement of the pile. The material from the drilled hole shall be removed and disposed of by the Contractor in an approved site.

Particular care must be taken in the drilling operation to avoid deflecting the bit along a sloping bedrock/shale line. To verify proper alignment, the Contractor shall measure and record the vertical alignment of the hole using a plumb bob or other acceptable method.

Preferably, the diameter of the drilled hole shall be a size that will allow the pile, while being slowly lowered into the hole, to reach the bottom of the hole under the impetus of the pile weight. The minimum hole diameter shall be 2 inches (50 mm) larger than the diagonal distance across the pile cross section.

Light tapping (ten blows with at least 3 inches (75 mm) of penetration per blow) with a pile hammer exerting no more than 12,000 ft/lbs (16 kJ) of energy is permitted at the direction of the Engineer to advance the pile past minor obstacles in the hole.

Temporary casing of holes may be needed to maintain an open clean hole through the soil overburden. There will be no additional compensation for temporary casing. The cost of any casing used shall be included in the unit price bid for piling.

#### 614.4 - INSTALLATION OF PILES:

Piles shall be located as shown on the Plans or as directed by the Engineer. Piles shall be installed with the pile center within 1 inch (25 mm) of the Plan location. The piles must be prevented from rotating, so that the pile axis is within five degrees of the position shown on the Plans.

The maximum permissible vertical deviation for piles shall be one percent of the total pile length, as measured at the actual pile location.

It is desirable that piles be installed without splicing; however, at the direction of the Engineer splices may be made. Splice lengths at the top of the piles may be butt welded provided the splice lengths are less than the required splice plates. No payment will be made for cut-offs. Welding shall be in accordance with 615.3.16.

Accurate records shall be maintained by the Contractor showing the depth to which each pile was placed, the plumbness, the amount of material used, elevation of bedrock/shale, and any unusual conditions encountered during the pile installation. These records shall be incorporated into the permanent records of the project.

#### 614.5 - CORROSION PROTECTION:

Piles will be protected from corrosion and sealed by the placement of

concrete or grout, from the bottom of the hole to the bottom of the lagging or as directed by the Engineer. Vibration of the concrete or grout is not required. The Contractor shall complete all concrete or grout operations for holes drilled during the work day.

The drilled hole shall be pumped free of water and shall be reasonably free of fall-in soil or other debris prior to the placement of the concrete or grout. The concrete or grout in the bedrock/shale portion of the hole will be pumped or tremied through a pipe beginning at the bottom of the drilled hole. The pipe shall be slowly raised ensuring the pipe end remains at least 2 feet (600 mm) below the surface of the concrete or grout. A means of positively measuring the elevation of the concrete or grout as it is placed shall be provided by the Contractor.

After placing the concrete or grout in the bedrock/shale, the Contractor has the option of either pumping or pouring directly into the hole the remainder of the concrete or grout. Placing the concrete or grout from the bottom of the hole to the bottom of the lagging shall be accomplished in one continuous operation.

The Contractor will inform the Engineer, at the preconstruction conference, as to the type of corrosion protection that will be used. Intermixing of concrete and grout will not be allowed, unless approved by the Engineer.

Concrete shall be in accordance with Section 601, Class B. The job site testing is waived

Grout will be furnished and placed in accordance with the requirements specified herein.

The acceptance sampling and testing of the grout is the responsibility of the Division.

Quality Control of the concrete or grout is there responsibility of the Contractor as designated in Materials Procedure MP 601.03.50. The Contractor shall maintain equipment and qualified personnel, who shall direct all field inspection, sampling, and testing necessary to determine the magnitude of the various properties of the concrete and grout governed by the Specifications and shall maintain these properties within the limits of this Specification. The Quality Control Plan designated in MP 601.03.50 shall be submitted to the Engineer at the pre construction conference. Work shall not begin until the Plan is reviewed for conformance with the contract documents.

The required 7-day compression strength of the grout shall be a minimum of 2,000 psi (14 MPa). Grout which does not attain the 2,000 psi (14 MPa) strength in 7 days but exceeds a strength of 1,600 psi (11 MPa) shall be subject to price reduction based on the percentage of strength attained.

A grout strength test shall consist of testing three 6 in x 12 in (150 mm x 300 mm) cylindrical specimens. The test results shall be the average of the three specimens. One set of three specimens shall be made for each day's operations.

The bid price for the piling with grout compressive strengths greater than or equal to 2,000 psi (14 MPa) will be paid at 100 percent unless the piling installation does not meet Specifications for other reasons. Between 1,600 psi

#### 614.6

(11 MPa) and 2,000 psi (14 MPa) compressive strengths, the cost of the grout will be deducted from the actual grout cost on a proportional basis with 2,000 psi (14 MPa) being 100 percent and 1,600 psi (11 MPa) being zero percent payment. With 1,600 psi (11 MPa) grout, the piling installation would be considered to meet 80 percent of the Specifications and the penalty being zero payment for the grout.

The penalty would involve only the quantity of grout represented by the

failing compressive strength results.

The bid price for the piling will be reduced for the piles grouted with grout having less than 1,600 psi (11 MPa) compressive strengths as follows:

A = Compressive strength of grout

B = Total foot (meter) of piling grouted with

C = Unit bid price per foot (meter) of piling

D = Cost of grout (from Contractor)

E = 2,000 psi (14 Mpa)

F = Total penalty

F = D + [BC - D] 10.80 - (A + E)

#### 614.6 - PAINTING:

All surfaces from the top of the steel pile, down to and including 2.0 ft. (600 mm) below the top of the anticipated grout line shall be cleaned and painted. The method of surface preparation shall be hand tool cleaning to SSPC-SP-2. The paint system shall consist of one-coat of aluminum epoxy mastic meeting the requirements of 711.12 applied at a minimum dry film thickness of 5 mils (125  $\mu$ m).

#### 614.7 - LAGGING AND BACKFILLING:

Lagging of the type and size as specified on the Plans shall be installed between the piles. Backfilling and restoration of the roadway template shall be as shown on the Plans.

Timber lagging shall be Grade #3 or better treated rough cut oak, 3 in (75 mm) wide by 8 in (200 mm) deep for heights up to 11 ft (3.4 m); and for wall heights exceeding 11 ft (3.4 m) the timber lagging shall be double 3 in (75 mm) wide by 8 in (200 mm) deep. The boards shall be cut to their required length prior to preservative treatment.

The timber lagging shall conform to Sections 710.3 and 710.4 of the West Virginia Division of Highways Standard Specifications and shall be CCA treated for soil and fresh water use, as per AWPA C2.

#### 614.8 - METHOD OF MEASUREMENT:

The quantity of piles will be measured in linear feet (meters) of piles installed and accepted for the wall. The quantity of lagging will be measured in square feet (meters) of lagging installed and accepted for the wall.

#### 614.9-BASIS OF PAYMENT:

The quantities will be paid for at the contract unit prices bid for the items listed below, which prices and payments shall be full compensation for furnishing all materials and doing all the work herein prescribed in a workmanlike and acceptable manner, including all labor, tools, equipment, supplies, and incidentals necessary to complete the work. The cost of drilling, concrete, grout, wales, and painting shall be included in the price bid for the piles. The cost of painting and welding steel lagging shall be included in the price bid for steel lagging.

#### 614.10-PAY ITEMS:

ITEM	DESCRIPTION	UNIT
614001-*	"size" STEEL PILE	LINEAR FOOT (METER)
614002-*	STEEL LAGGING, THICKNESS "thickness"	SQUARE FOOT (METER)
614003-*	CONCRETE LAGGING, THICKNESS "thickness"	SQUARE FOOT (METER)
614004-*	TIMBER LAGGING	SQUARE FOOT (METER)

^{*} Sequence number



# SUPPLEMENTARY SPECIFICATIONS

The specifications for project shall be the West Virginia Department of Highways, "Standard Specifications, Roadways and Bridges", except as modified herein.

#### 614.1 - DESCRIPTION

Delete the last sentence in the first paragraph

#### 614.3 - DRILLING

Revise the second paragraph to read the following:

"The minimum embedment of the pile length into bedrock shall be designated on the plans. The total estimated pile length and depth to estimated bedrock are shown in the 'Drilled Shaft Schedule' and on the 'Drilled Shaft Profile'. Should the actual elevation vary by more than 2.5 feet, the Engineer must approve the hole prior to placement of the pile. The material from the drilled hole shall be removed and disposed of by the Contractor in an approved site."

Revise portions of the fourth paragraph to state the minimum diameter of the drilled hole shall be as shown on the plans.

# 614.5 - CORROSION PROTECTION

Revise the first paragraph to indicate that vibration of concrete will be required for the upper ten feet of the drilled shaft.

Delete from paragraph 11 to the end of section 614.5 inclusively. All concrete is expected to reach a minimum 7 day strength of 1,600 psi and 28 day strength of 4,000 psi.

#### 614.6 - PAINTING

Delete this section in its entirety. Painting of the structural steel is not required.

## 614.7 - LAGGING AND BACKFILLING

Delete paragraphs two and three in this section. Timber lagging is not applicable to this project.

#### MEASUREMENT AND PAYMENT

## LINE ITEM 1 - Mobilization and Demobilization

This item will cover the payment for the mobilization and demobilization of all plant and equipment to execute the project. Payment will be made on a LUMP SUM basis.

#### LINE ITEM 2 - Clear & Grub

This item will cover the payment for clearing and grubbing the area of work, of all trees, shrubs, etc. Payment will be made on a PER ACRE basis.

# LINE ITEM 3 - Excavation and Embankment

This item will cover payment for all excavation and embankment not otherwise included in any other line items on the Bid Form. These items include, but are not limited to, excavation and grading in front of the drilled shaft wall, excavation for the precast concrete lagging, any and all benching that may be required, miscellaneous backfill that may be required, etc. Payment will be made on a LUMP SUM basis.

LINE ITEM 4 - Erosion & Sediment Control

This item will cover all erosion and sediment control measures incorporated by the Contractor's approved SWPPP. Payment will be made on a LUMP SUM basis.

#### LINE ITEM 5 - Roadway Grading

This item will cover the scariffying, mixing and recompacting of the upper 12" of the existing roadbed. Payment for this item will be made on a per CUBIC YARD basis.

#### LINE ITEM 6 - Stone Base for Road

This item will cover the 12" of ABC crushed stone to be placed for the new roadway. Payment for this item will be made on a per TON basis.

#### LINE ITEM 7 – 30" Diameter Drilled Shafts, Above Bedrock

This item will cover all means, methods and materials to perform the machine excavation of the material above bedrock at the drilled shaft locations. Also included in this item shall be the concrete fill material, spoil removal, and forming above grade if necessary. Payment for this item will be made on a per LINEAR FOOT basis.

#### LINE ITEM 8 – 30" Diameter Drilled Shafts, Into Bedrock

This item will cover all means, methods and materials to perform the machine excavation of the material into bedrock at the drilled shaft locations. Also included in this item shall be the concrete fill material and spoil removal. Payment for this item will be made on a per LINEAR FOOT basis.

#### LINE ITEM 9 – Steel Piles, W18x106

This item will cover all means, methods and materials to furnish, fabricate and place the steel piles at the proper centerline locations indicated. Payment for this item will be made on a per LINEAR FOOT basis.

#### LINE ITEM 10 - Steel Piles, W21x111

This item will cover all means, methods and materials to furnish, fabricate and place the steel piles at the proper centerline locations indicated. Payment for this item will be made on a per LINEAR FOOT basis.

#### LINE ITEM 11 - Precast Concrete Lagging, 8" Thick

This item will cover all means, methods and materials to furnish, fabricate and place the 8" thick precast concrete lagging at the proper locations indicated. Payment for this item will be made on a per SQUARE FOOT basis.

#### LINE ITEM 12 – Free Draining Backfill

This item will cover all excavation, furnishing & installation of backfill, compaction necessary and furnishing & installation filter fabric. Payment for this item will be made on a per TON basis.

#### LINE ITEM 13 – 6" Perforated Pipe

This item will cover furnishing & installation of the 6" perforated pipe embedded in the free draining backfill. Payment for this item will be made on a per LINEAR FOOT basis.

#### LINE ITEM 14 - Cable Guardrail

This item will cover all means, methods and materials to furnish, fabricate and place the cable guardrail at the proper locations indicated. Payment for this item will be made on a per LINEAR FOOT basis.

#### LINE ITEM 15 - Seed

This item will cover all means, methods and materials to furnish and place the seeding over all areas disturbed. Payment for this item will be made on a per ACRE basis.

\$331,17679	D	TOTAL BID			
\$ 1,49158	23983.16	ACRE	0.50	Seed	17
\$ 20,56600	73.45		280.00	Cable Guardrail	16
\$ 1,04420	4.54	듸	230.00	6" Perforated Pipe	15
\$ 6,348.700	42.32	TON	150.00	Free Draining Backfill	14
\$ 36,61830	17.69	SFT	2,070.00	Precast Concrete Lagging, 8" Thick	13
\$ 32,54000	162,70	딬	200.00	Steel Piles, W21x111	12
\$135,99320	199.99	딥	680.00	Steel Piles, W18x106	
\$22,575 60	62.71	되	360.00	30" Diameter Drilled Shafts, Into Bedrock	10
	55.69	딬	481.57	30" Diameter Drilled Shafts, Above Bedrock	9
\$ 1,39600	34.90	S	40.00	Soil Boring	8
\$ 6,06288	22.29	TON	272.00	Stone Base for Road	7
\$ 1,55448	11.43	CY	136.00	Roadway Grading	6
\$ 5,628. ~60	5,628.60	LS	1.00	Construction Layout & Staking	<b>ა</b>
\$ 5,698 59	5,698.59	LS	1.00	Erosion & Sediment Control	4
\$ 7,84804	7,848.04	LS	1.00	Excavation and Embankment	ω
\$ 7.848 - 04	31,392.16	ACRE	0.25	Clear & Grub	2
\$11,144 - 65	11,144.65	LS	1.00	Mobilization and Demobilization	`
Amount	Unit Price	Unit	Quantity	Description	Bid Item Number

Title:	Print Name:	Signed:	Date:		Address:	Contractor Name:
Title: Vice President	Print Name: Atelio F. Weale	HalitMerle	4/21/09	Morgantown, WV 26501	Address: 302 Dents Run Road	Contractor Name: Laurita Excavating, Inc.

#### **BID BOND**

KNOW ALL M	IEN BY THESE PRE	ESENTS, That we, the	e undersigned,La	aurita Exca	<u>ivating, Inc</u>	*
of Morgani	town	, <u>WV</u>		_, as Princip	al, and <u>Fidel</u>	lity and Deposit Company of
Maryland of Baltimor	e ,MD		a corporation or	ganized and	l existing und	der the laws of the State of
MD with	its principal office in	the City of <u>Baltimor</u>	re	, as Surety	, are held an	d firmly bound unto the State
of West Virginia, as Ob	ligee, in the penal si	um of <u>Five Percent c</u>	of Amount Bid	(\$	5%	) for the payment of which,
well and truly to be made	de, we jointly and se	verally bind ourselve	s, our heirs, admi	inistrators, e	executors, su	iccessors and assigns.
The Condition	of the above obligation	tion is such that wher	eas the Principal	has submit	ted to the Pu	rchasing Section of the
Department of Adminis	tration a certain bid	or proposal, attached	hereto and made	e a part here	eof, to enter	into a contract in writing for
Reclamation - Resto						· · · · · · · · · · · · · · · · · · ·
NOW THERE	FORE.					
	shall be rejected, or	r				
(b) If said bid	shall be accepted a	and the Principal shall				the bid or proposal attached
						her respects perform the sobligation shall remain in full
						s hereunder shall, in no event,
exceed the penal amou	int of this obligation	as herein stated.	•			
The County in	with a control recently and	harahir ofinistotaa mm	d agrapa that the	oblications	of anid Cure	ety and its bond shall be in no
rne Surety, to wav impaired or affecte	r the value received, d by any extension (	, nereby slipulates an of the time within whic	ch the Obligee m	ay accept si	uch bid, and	said Surety does hereby
waive notice of any suc			_	. ,		•
	AUTOFOE Osinala	al and Dissolutions to	de simula and dhair h	anda and a	ania ama au	ah af tham so are carparations
		,				ch of them as are corporations
have caused their corpo			ise presents to be	signed by	their proper	OTTICETS, INIS
15th day of	Aprii	, 2009				
				Lourito Ev	cavating, In	20
Principal Corporate Sea	31			Launta Ex		e of Principal)
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Surety Corporate Seal	130 OKA	Cosin		Fidelity an		Company of Maryland e of Surety)
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	A Comment	LAND SEE			ndeison Attor	ney-in-Fact

IMPORTANT – Surety executing bonds must be licensed in West Virginia to transact surety insurance. Corporate seals must be affixed, and a power of attorney must be attached.

#### **Power of Attorney** FIDELITY AND DEPOSIT COMPANY OF MARYLAND

KNOW ALL MEN BY THESE PRESENTS: That the FIDELITY AND DEPOSIT COMPANY OF MARYLAND, a corporation of the State of Maryland, by THEODORE G. MARTINEZ, Vice President, and ERIC D. BARNES, Assistant Secretary, in pursuance of authority granted by Article VI, Section 2, of the By-Laws of said Company, which are set forth on the reverse side hereof and are hereby certified to be in full force and effect on the date hereof does hereby nominate, constitute and appoint Karen M. COMPORT, Wendy L. PRICE, Zachary D. MENDELSON, Mark F. SUSCO and Kathryn J. FINDLAY, all of Pittsburgh, Pennsylvania, EACH is much and lawful agont and automey-in-Fact, to make, execute, seal and deliver, for, and on its behalf as surery and as its aer and deed any and all bonds and undertakings, and the execution of such bonds or undertakings impursuance of these presents that be as binding upon said Company, as fully and amply, to all intents and purposes as if they had been different and acknowledged by the regularly elected officers of the Company at its office in Ballimore, Md, in their own proper persons. This power of attorney revokes that issued on be to the company at its office in Ballimore, Md, in their own proper persons. behalf of Karen M. COMPORT, World PRICE, Zachary L. MENDELSON, Mark F. SUSCO, dated May 12, 2005.

The said Assistant Secretary does hereby certify that the extract set forth on the reverse side hereof is a true copy of Article VI, Section 2, of the By-Laws of said Company, and is now in force.

IN WITNESS WHEREOF, the said Vice-President and Assistant Secretary have hereunto subscribed their names and affixed the Corporate Seal of the said FIDELITY AND DEPOSIT COMPANY OF MARYLAND, this 27th day of June, A.D. 2007.

fine D. Barry

ATTEST:

FIDELITY AND DEPOSIT COMPANY OF MARYLAND



Eric D. Barnes Assistant Secretary

Theodore G. Martinez

Theken A Wanting

State of Maryland City of Baltimore ss:

On this 27th day of June, A.D. 2007, before the subscriber, a Notary Public of the State of Maryland, duly commissioned and qualified, came THEODORE G. MARTINEZ, Vice President, and ERIC D. BARNES, Assistant Secretary of the FIDELITY AND DEPOSIT COMPANY OF MARYLAND, to me personally known to be the individuals and officers described in and who executed the preceding instrument, and they each acknowledged the execution of the same, and being by me duly sworn, severally and each for himself deposeth and saith, that they are the said officers of the Company aforesaid, and that the seal affixed to the preceding instrument is the Corporate Seal of said Company, and that the said Corporate Seal and their signatures as such officers were duly affixed and subscribed to the said instrument by the authority and direction of the said Corporation.

IN TESTIMONY WHEREOF, I have hereunto set my hand and affixed my Official Seal the day and year first above written.

Constance A. Dunn

Notary Public

My Commission Expires: July 14, 2011

Constance a. Dunn

#### EXTRACT FROM BY-LAWS OF FIDELITY AND DEPOSIT COMPANY OF MARYLAND

"Article VI, Section 2. The Chairman of the Board, or the President, or any Executive Vice-President, or any of the Senior Vice-Presidents or Vice-Presidents specially authorized so to do by the Board of Directors or by the Executive Committee, shall have power, by and with the concurrence of the Secretary or any one of the Assistant Secretaries, to appoint Resident Vice-Presidents, Assistant Vice-Presidents and Attorneys-in-Fact as the business of the Company may require, or to authorize any person or persons to execute on behalf of the Company any bonds, undertaking, recognizances, stipulations, policies, contracts, agreements, deeds, and releases and assignments of judgements, decrees, mortgages and instruments in the nature of mortgages,...and to affix the seal of the Company thereto."

#### **CERTIFICATE**

I, the undersigned, Assistant Secretary of the FIDELITY AND DEPOSIT COMPANY OF MARYLAND, do hereby certify that the foregoing Power of Attorney is still in full force and effect on the date of this certificate; and I do further certify that the Vice-President who executed the said Power of Attorney was one of the additional Vice-Presidents specially authorized by the Board of Directors to appoint any Attorney-in-Fact as provided in Article VI, Section 2, of the By-Laws of the FIDELITY AND DEPOSIT COMPANY OF MARYLAND.

This Power of Attorney and Certificate may be signed by facsimile under and by authority of the following resolution of the Board of Directors of the FIDELITY AND DEPOSIT COMPANY OF MARYLAND at a meeting duly called and held on the 10th day of May, 1990.

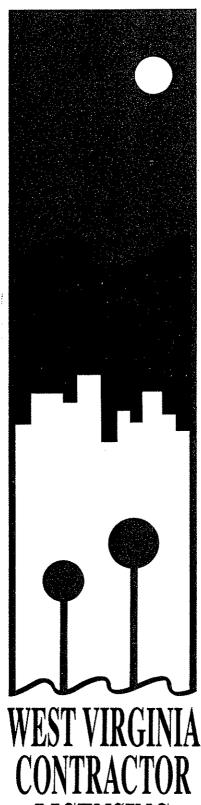
RESOLVED: "That the facsimile or mechanically reproduced seal of the company and facsimile or mechanically reproduced signature of any Vice-President, Secretary, or Assistant Secretary of the Company, whether made heretofore or hereafter, wherever appearing upon a certified copy of any power of attorney issued by the Company, shall be valid and binding upon the Company with the same force and effect as though manually affixed."

IN TESTIMONY WHEREOF, I have hereunto subscribed my name and affixed the corporate seal of the said Company,

	CyceE.1	yun-
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15 day of April

Assistant Secretary



# **CONTRACTOR LICENSE**

Authorized by the

# West Virginia Contractor Licensing Board

Number:

WV001135

#### Classification:

GENERAL BUILDING
GENERAL ENGINEERING
MULTIFAMILY
RESIDENTIAL
SPECIALTY
EXCAVATION

LAURITA EXCAVATING INC 302 DENTS RUN ROAD MORGANTOWN, WV 26507

## **Date Issued**

# **Expiration Date**

AUGUST 13, 2008 AUGUST 13, 2009

Authorized Company Signature

Chair, West Virginia Contractor
Licensing Board

This license, or a copy thereof, must be posted in a conspicuous place at every construction site where work is being performed. This license number must appear in all advertisements, on all bid submissions and on all fully executed and binding contracts. This license cannot be assigned or transferred by licensee. Issued under provisions of West Virginia Code, Chapter 21, Article 11.





# State of West Virginia DRUG FREE WORKPLACE CONFORMANCE AFFIDAVIT West Virginia Code §21-1D-5

STATE OF West Virginia
COUNTY OF Monongalia, TO-WIT:
I, Atlio F. Meale, after being first duly sworn, depose and state as follows:
1. I am an employee of <u>aurita Excavating</u> , In.; and, (Company Name)
2. I do hereby attest that Laurita Excavating, Inc. (Company Name)
maintains a valid written drug free workplace policy and that such policy is in compliance with <b>West Virginia Code</b> §21-1D-5.
The above statements are sworn to under the penalty of perjury.
Laurita Excavating, Inc. (Company Name)
By: Atela 7 Meals
Title: Vice President
Date: 4/21/09
Taken, subscribed and sworn to before me this 21 day of April 2009
By Commissional Spirites February 15,2015
Notary Public, State of West Virginia  BRANDI S. GIBSON  Route 1 Box 68  Masontown, WV 26542
My commission expires February 15, 2015 (Notary Public)
and the second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second s

THIS AFFIDAVIT MUST BE SUBMITTED WITH THE BID IN ORDER TO COMPLY WITH WV CODE PROVISIONS. FAILURE TO INCLUDE THE AFFIDAVIT WITH THE BID SHALL RESULT IN DISQUALIFICATION OF THE BID.

Rev March 2009

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## STATE OF WEST VIRGINIA Purchasing Division

# **PURCHASING AFFIDAVIT**

#### VENDOR OWING A DEBT TO THE STATE:

West Virginia Code §5A-3-10a provides that: No contract or renewal of any contract may be awarded by the state or any of its political subdivisions to any vendor or prospective vendor when the vendor or prospective vendor or a related party to the vendor or prospective vendor is a debtor and the debt owed is an amount greater than one thousand dollars in the aggregate.

#### PUBLIC IMPROVEMENT CONTRACTS & DRUG-FREE WORKPLACE ACT:

If this is a solicitation for a public improvement construction contract, the vendor, by its signature below. affirms that it has a written plan for a drug-free workplace policy in compliance with Article 1D, Chapter 21 of the West Virginia Code. The vendor must make said affirmation with its bid submission. Further, public improvement construction contract may not be awarded to a vendor who does not have a written plan for a drug-free workplace policy in compliance with Article 1D, Chapter 21 of the West Virginia Code and who has not submitted that plan to the appropriate contracting authority in timely fashion. For a vendor who is a subcontractor, compliance with Section 5, Article 1D, Chapter 21 of the West Virginia Code may take place before their work on the public improvement is begun.

#### ANTITRUST:

In submitting a bid to any agency for the state of West Virginia, the bidder offers and agrees that if the bid is accepted the bidder will convey, sell, assign or transfer to the state of West Virginia all rights, title and interest in and to all causes of action it may now or hereafter acquire under the antitrust laws of the United States and the state of West Virginia for price fixing and/or unreasonable restraints of trade relating to the particular commodities or services purchased or acquired by the state of West Virginia. Such assignment shall be made and become effective at the time the purchasing agency tenders the initial payment to the bidder.

I certify that this bid is made without prior understanding, agreement, or connection with any corporation, firm, limited liability company, partnership or person or entity submitting a bid for the same materials, supplies, equipment or services and is in all respects fair and without collusion or fraud. I further certify that I am authorized to sign the certification on behalf of the bidder or this bid.

#### LICENSING:

Vendors must be licensed and in good standing in accordance with any and all state and local laws and requirements by any state or local agency of West Virginia, including, but not limited to, the West Virginia Secretary of State's Office, the West Virginia Tax Department, West Virginia Insurance Commission, or any other state agencies or political subdivision. Furthermore, the vendor must provide all necessary releases to obtain information to enable the Director or spending unit to verify that the vendor is licensed and in good standing with the above entities.

#### CONFIDENTIALITY:

1

The vendor agrees that he or she will not disclose to anyone, directly or indirectly, any such personally identifiable information or other confidential information gained from the agency, unless the individual who is the subject of the information consents to the disclosure in writing or the disclosure is made pursuant to the agency's policies, procedures and rules. Vendor further agrees to comply with the Confidentiality Policies and Information Security Accountability Requirements, set forth in http://www.state.wv.us/admin/purchase/privacy/ noticeConfidentiality.pdf.

Under penalty of law for false swearing (West Virginia Code §61-5-3), it is hereby certified that the vendor affirms and acknowledges the information in this affidavit and is in compliance with the requirements as stated. ١.

Vendor's Name: Laurita Excavating Ir	χ
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Authorized Signature: Atel. The lune	Date: 4-2/-09
Authorized Signature	
Durchneing Affidavit (Revised 01/01/09)	